

SPECIFYING SITE INVESTIGATION

Ir. Liew Shaw Shong



Introduction

Scope



- **Site Investigation**

- Information on Hydrology, Meteorology, Environment, Natural Resources, Activities & Topography

- **Ground Investigation**

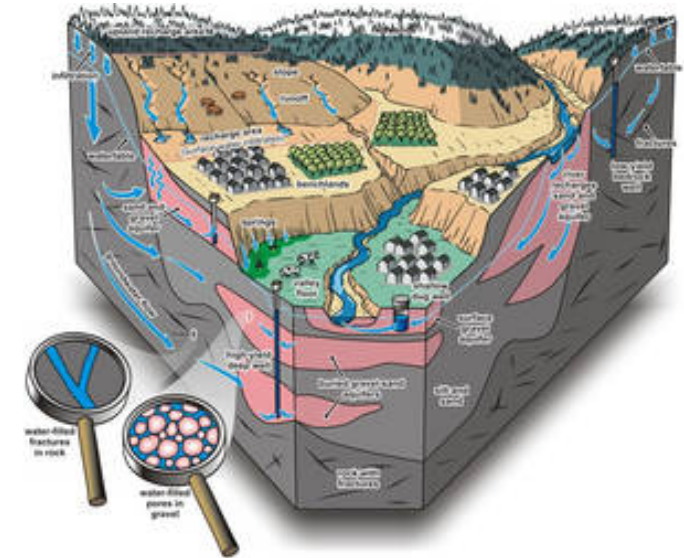
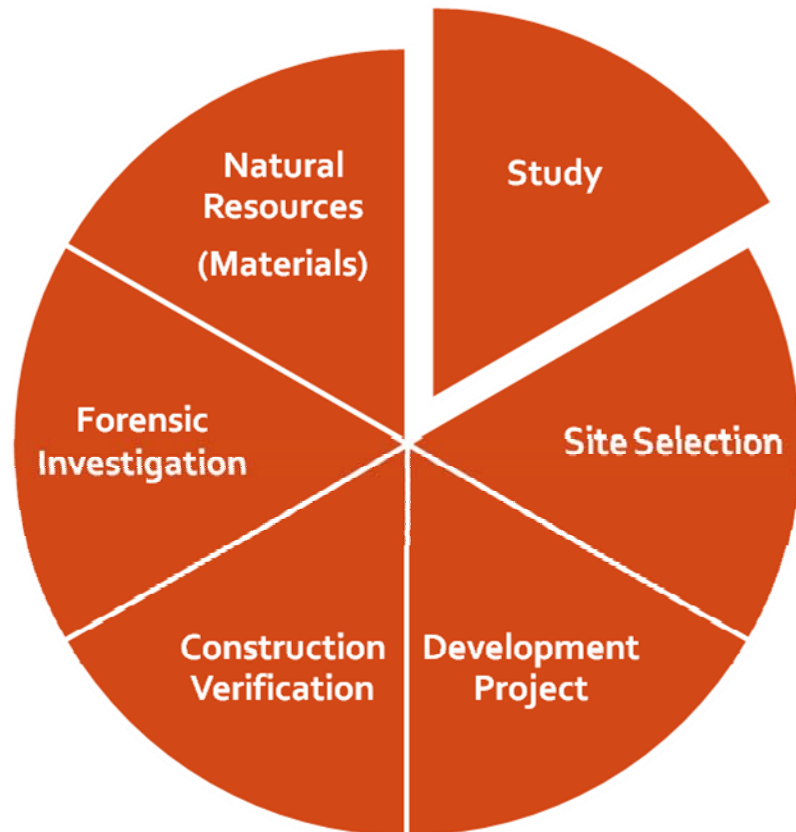
- Information on Ground & Groundwater conditions

- **Monitoring**

- Time dependent changes in ground movements, groundwater fluctuation & movements

Introduction

Purpose



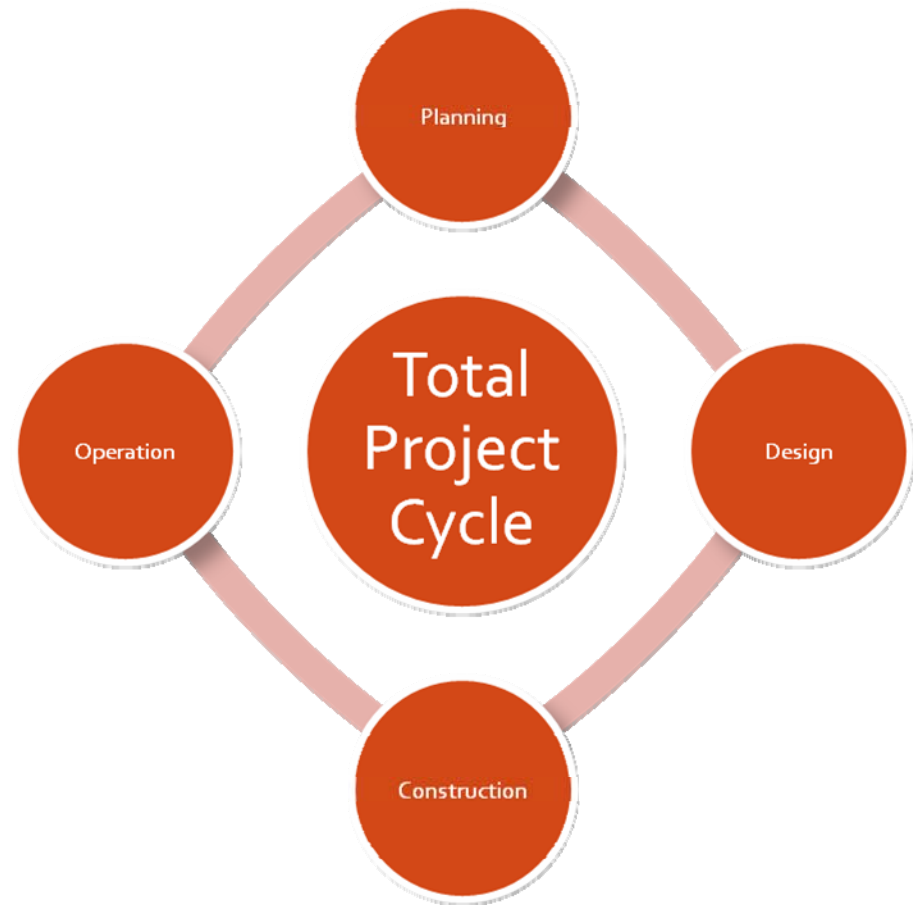
Introduction

Project Cycle

Ground Investigation

Common Problems & Trend

Conclusion



Why doing GI? Why Geotechnical Engineer? What Risk & Consequence

Why doing GI?

It is regarded as necessary, but not a rewarding expense. (Uncertainty, sufficiently accurate design options for Cost & Benefit study)



Why Geotechnical Engineer?

Geotechnical engineer as an underwriter for risk assessment.

What Risk in Ground & its Consequence ?

Ground Variability & Geo-hazards.

Financial Viability & Cost Overrun (Construction & Operation).



WITHOUT SI, GROUND IS AN HAZARD

Sink hole triggers dramatic Florida viaduct collapse

SITE INVESTIGATIONS failed to pick up a sink hole which caused a motorway viaduct to collapse in Tampa, Florida, last month.

Ground investigations involved borehole probes to 3.5m below the base of the 19.5m foundations for each of the viaduct's 212 piers.

Project client Tampa-Hillborough Expressway Authority said this was double normal requirements.

A 6m high pier for the 10km long highway sank suddenly into the ground on 13 April during construction of a glued segmental deck span.

The reinforced concrete pier almost completely disappeared.



The collapse was slow enough for workers to get clear, although two were taken to hospital. The busy Lee Roy Selman Crosstown Expressway which runs beneath the viaduct was closed until traffic could be redirected.

Cause of the collapse is thought to be a limestone sink

hole, more than 30m below the site. A spokeswoman for the Authority said Florida was largely underlain by limestone and sink holes were prevalent. It was impossible to determine the location of every one.

Ground investigation was by Dames & Moore, subcontractor to the Authority's general

engineering consultant URS. Additional input was made by Williams Earth Science.

Ground radar and seismic probing may now be used to check the remaining pier locations for the highway. The existing 160 piers should be okay said the Authority spokeswoman.

"The pier sank when the launching truss for assembling the 16 segment precast span was fully loaded which means it had half a 700,000lb (320t) load on it," she said. Existing piers have already received this de facto load test.

The \$310M project, due for completion next year, will create an additional three lanes, 6m above a busy existing commuter route. Traffic flow will reverse between morning and night.

Designer for the elevated structure is the Figg Engineering Group and contractor PCL Civil Constructors from Canada.

WITHOUT SI, GROUND IS AN HAZARD

Light saves man in sinkhole scare

IPOH: When Lee Pek Sang, 84, got up to answer nature's call at 5am yesterday, he realised something was amiss.

The toilet at his home in Bukit Mertah New Village, was missing — lost to a sinkhole.

The sinkhole, measuring 2.44m by 4.57m, was the 21st that had occurred in the area since last October, according to Geological Survey Department officers.

Lee said he would have fallen into the sinkhole if not for the light outside the toilet which was always switched on.

He also said he cannot leave his home because he had nowhere else to go.

The last sinkhole which appeared at the new village next to a cobbler's house on May 16 measured 2m by 8m.

MP for Batu Gajah Yeong Chee Wah will ask to make public the findings on the frequent appearance of sinkholes at the Bukit Merah new village during the Dewan Rakyat sitting in July.

Yeong said the affected villagers have been living in fear since October, adding that some form of remedial action should be taken.

At present, the sinkholes are a threat to 50 houses located side-by-side in three rows in the village.

The village elders when questioned about the occurrence of sinkholes there, said water from a nearby mining pool was drained away underground below the affected houses and could be the main reason for the sinkholes.

The Lahat mines about 500m away from the Bukit Merah New Village is a large and very deep dry mine which has existed for years and has been dubbed the "Grand Canyon of Malaysia" since it could be seen when travelling on the new Ipoh-Lumut outer ring road.

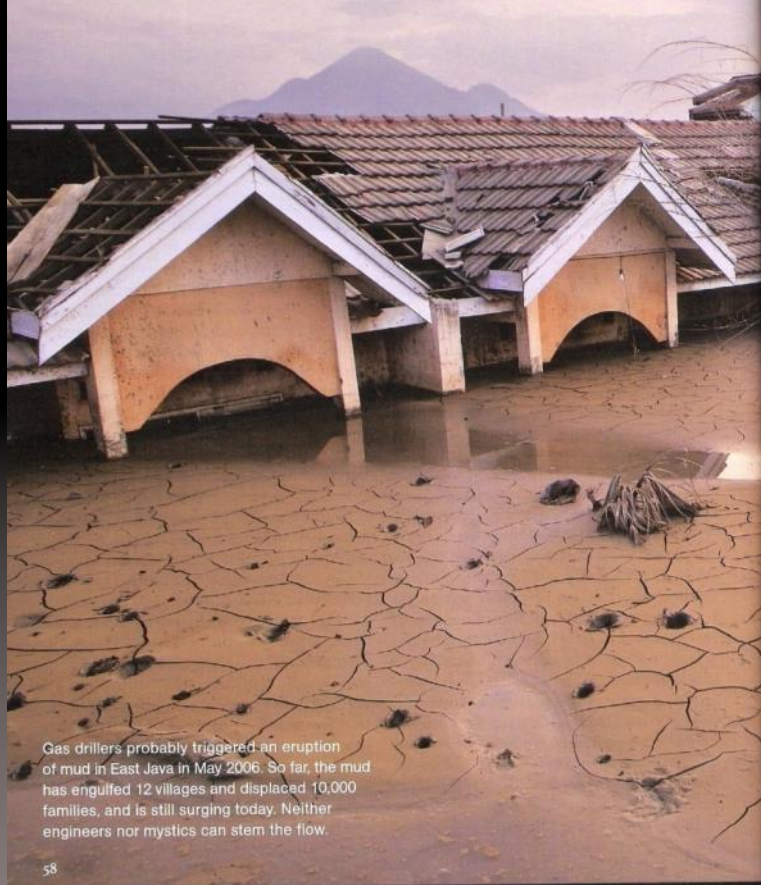


DEEP TROUBLE ... Yeong (standing, left) peering into the sinkhole at what used to be Lee's toilet yesterday. The MP pledges to follow up on the matter at the Dewan Rakyat in July.

WITH SI, GROUND CAN BE A HAZARD

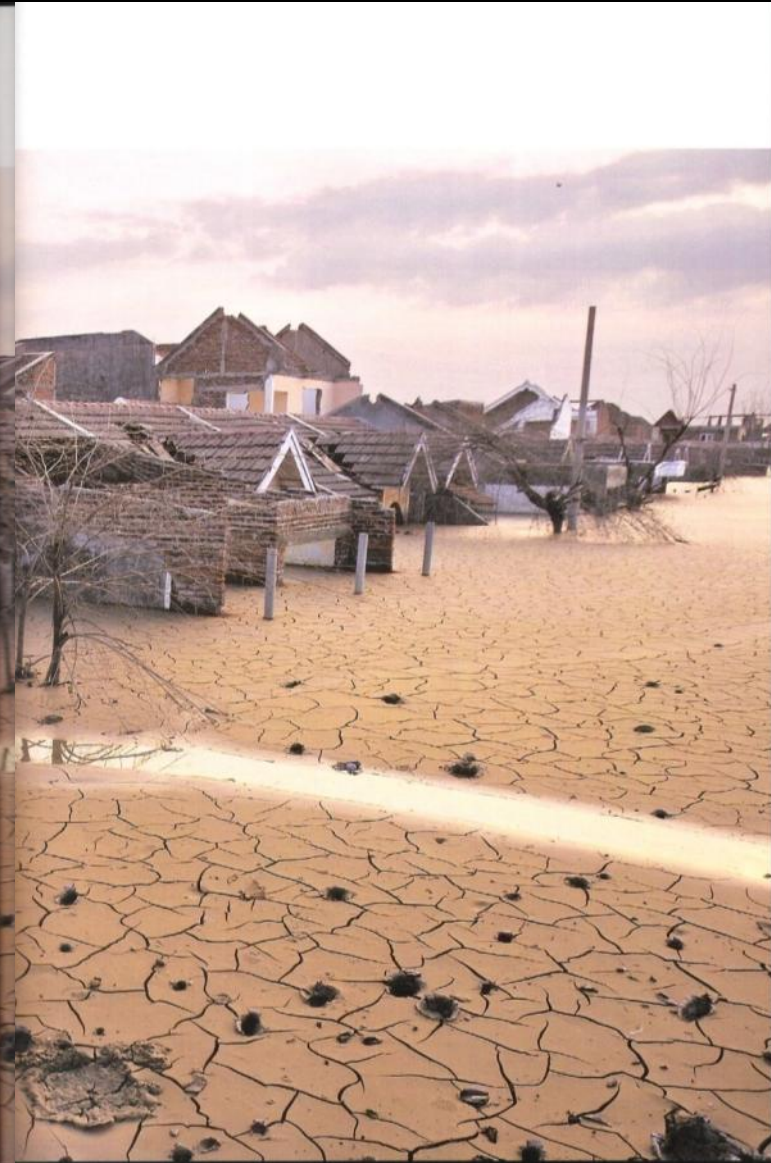
DROWNING IN MUD

AN UNNATURAL
DISASTER ERUPTS
WITH NO END
IN SIGHT



Gas drillers probably triggered an eruption of mud in East Java in May 2006. So far, the mud has engulfed 12 villages and displaced 10,000 families, and is still surging today. Neither engineers nor mystics can stem the flow.

58



Source : National Geographic (Jan 2008) ⁸

WITH SI, GROUND CAN BE A HAZARD

By Andrew Marshall
Photographs by John St

By dawn, the tr

to seep into the neighb
had become a scalding
the modest house bel
manages a store in the
Java. As it smothered fu
Sumitro and Indayani, l
and fled. "I knew the m
he says. "My house wa

Months later, a plum
a landscape of subme
source of his woes: a n
source of some controv
pany drilling for gas; ot
was the trigger.

Lusi, as Indonesias
one of the more bizarr
sia's geologic turmoil
spewed millions of b
blanketing an area
York City's Central P
peared under the mu
and 10,000 families
their homes. So far, a
mate, the catastrophe
billion dollars—nearly
and triggered spasms
being Indonesia, it ha
to the supernatural.

Lusi—a nickname
Indonesian word for
arjo, the name of the n
erupting for decades
backhoes work relentl
age, fortifying dikes a
of mud that continu
Pipes disgorge the
River; theoretically, r
it doesn't choke the
Surabaya, a city of 2.5

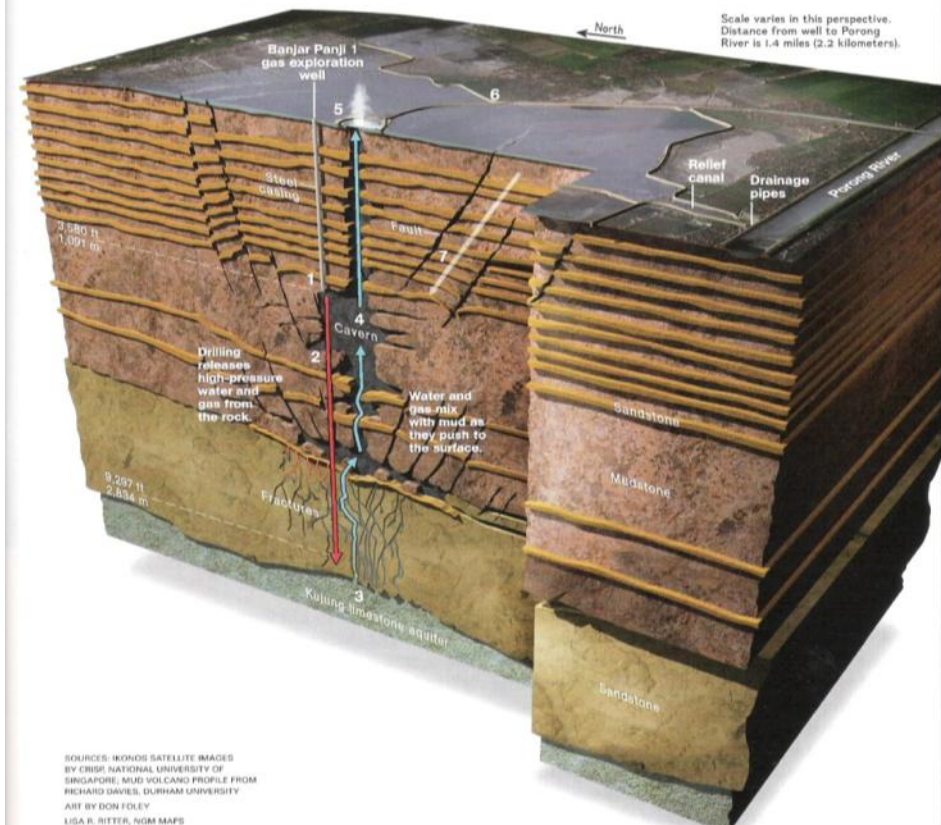
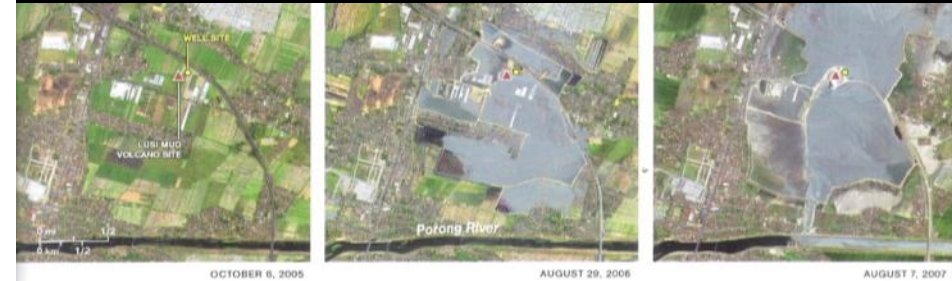
With the mud cam
witch doctors, Balin
celebrity soothsayer, l
they could stop the del
geese, and monkeys

60

UNDER THE MUD VOLCANO

It began with a burst of steam and a splurt of mud. But the gloopy surge that locals call Lusi soon became a sprawling nightmare. Satellite images (top) show Lusi swallowing more than two square miles in the Porong District. A cross section (right) illustrates what geologist Richard Davies believes caused the disaster.

1. Drillers exploring for gas bored 3,580 feet down, then inserted a steel casing to strengthen the hole.
2. Drilling went deeper without the steel casing. Water and gas filled the hole, and the resulting pressure fractured unprotected rock strata.
3. Hot, high-pressure water was released, probably from the Kujung aquifer.
4. The water raced upward and liquefied masses of mudstone.
5. Mud surged through layers of mudstone and sandstone and broke through the surface.
6. Engineers built dikes in an attempt to contain the mud.
7. Underground, caverns formed and collapsed, causing faults.



SOURCES: IKONOS SATELLITE IMAGES
BY CHRIS, NATIONAL UNIVERSITY OF
SINGAPORE; MUD VOLCANO PROFILE FROM
RICHARD DAVIES, DURHAM UNIVERSITY
ART BY DON FOLEY
LISA R. RITTER, NDM MAPS

Source :National Geographic (Jan 2008) 9

GOD CREATION & HUMAN CREATION



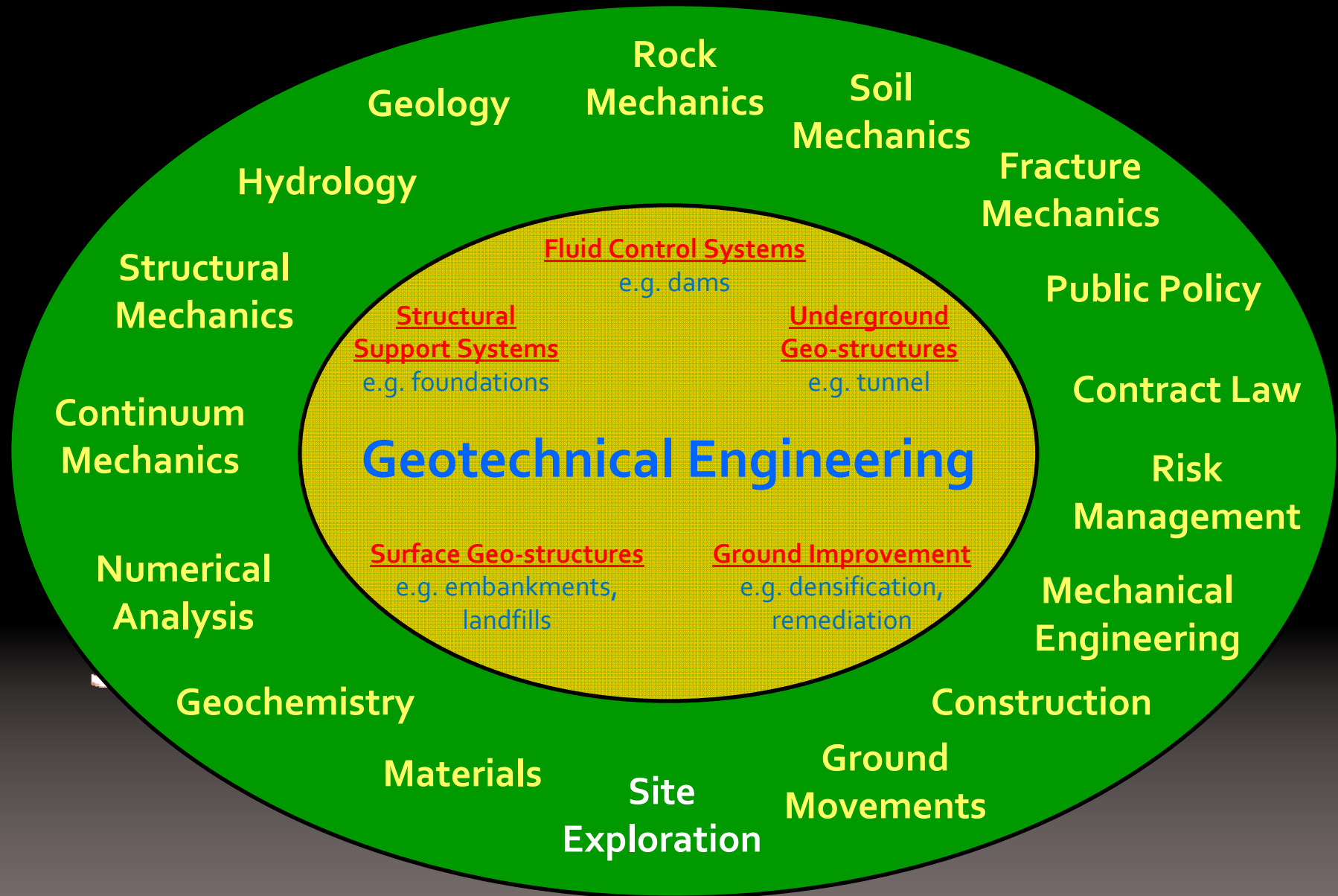
Elements to deal with Uncertainties

Pertinent
Knowledge

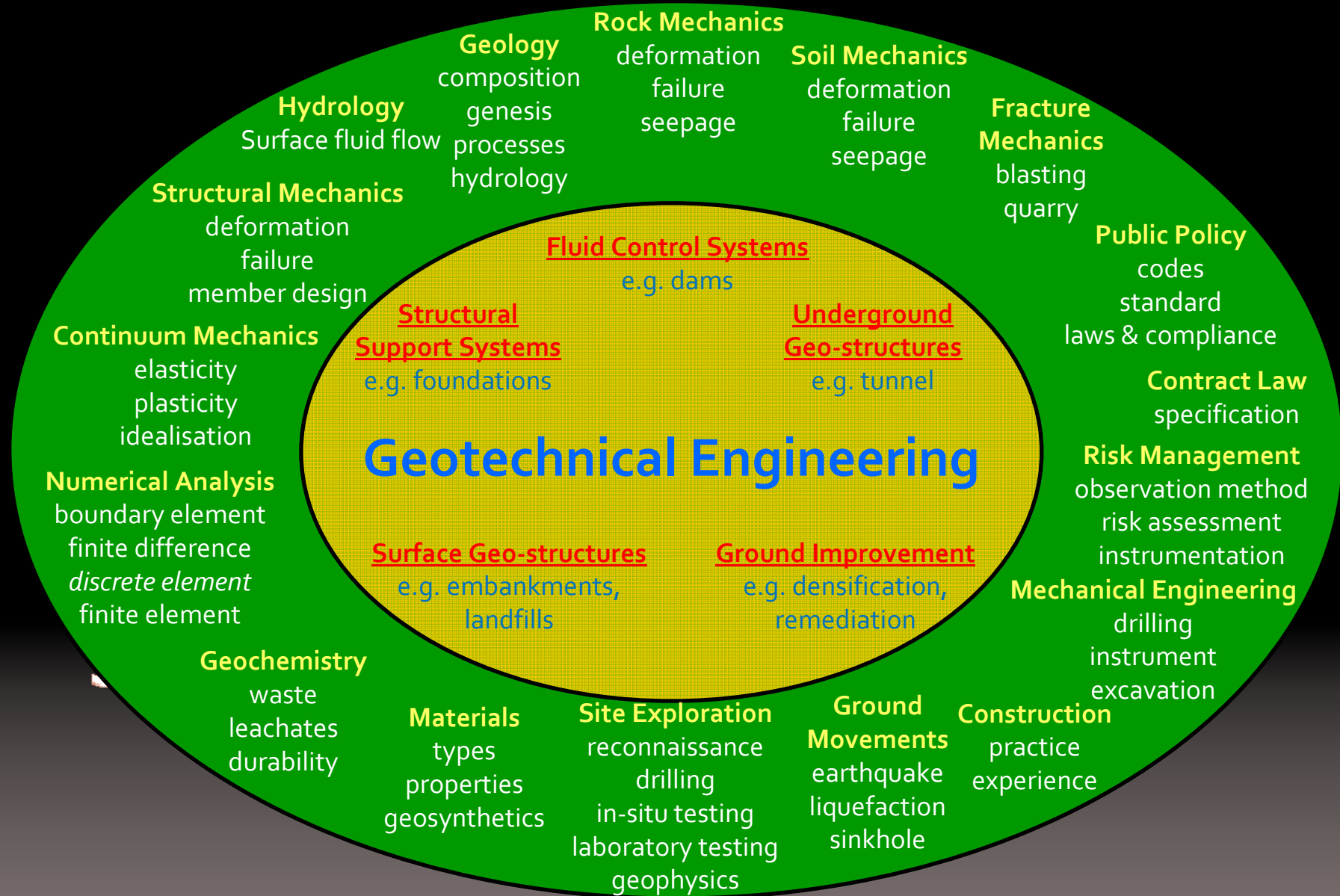
Observation
& Experience

Information

Careful
Interpretation
&
Assessment



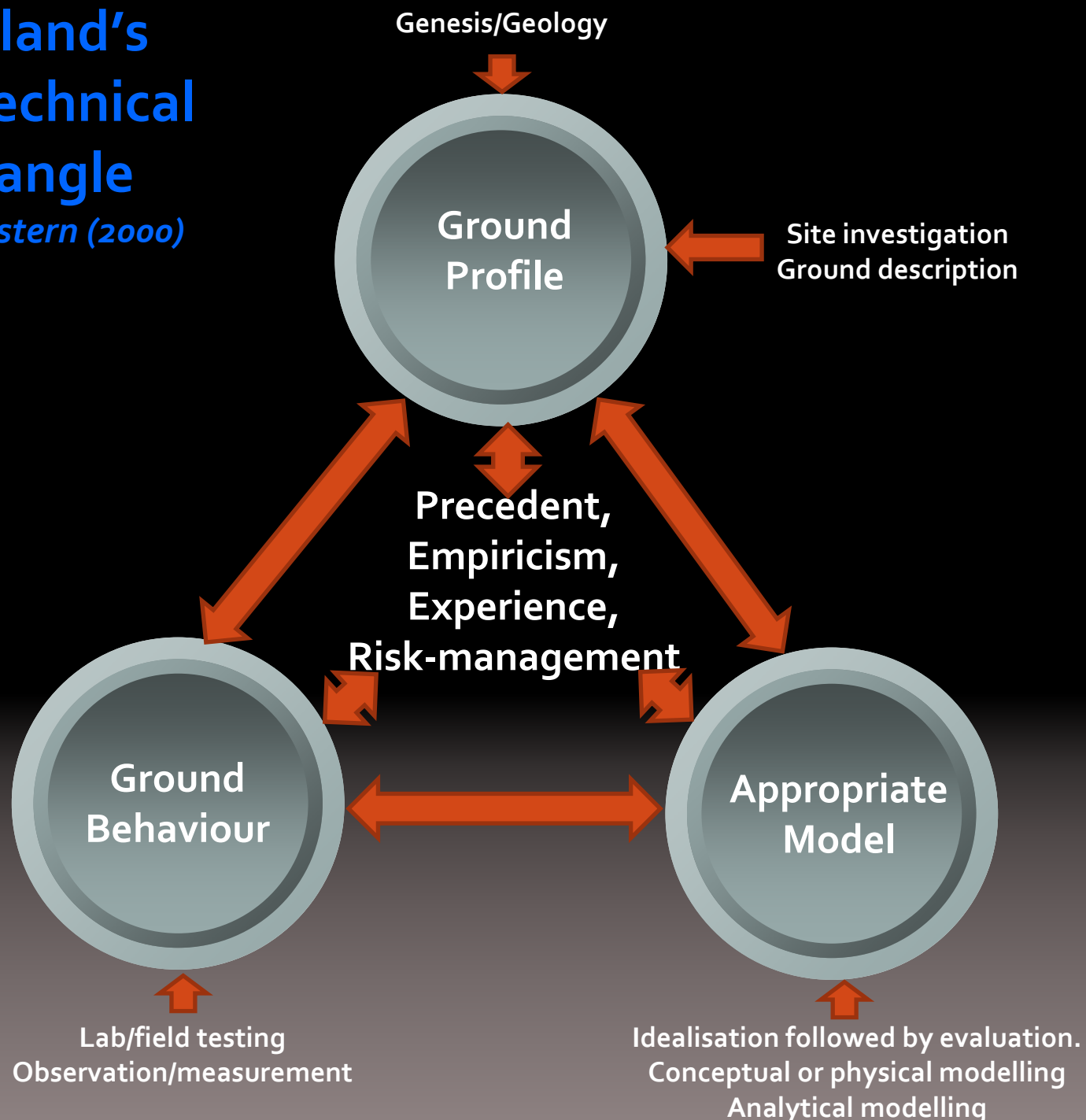
Modified from Morgenstern (2000)



Modified from Morgenstern (2000)

Burland's Geotechnical Triangle

Morgenstern (2000)



■ How GI cost

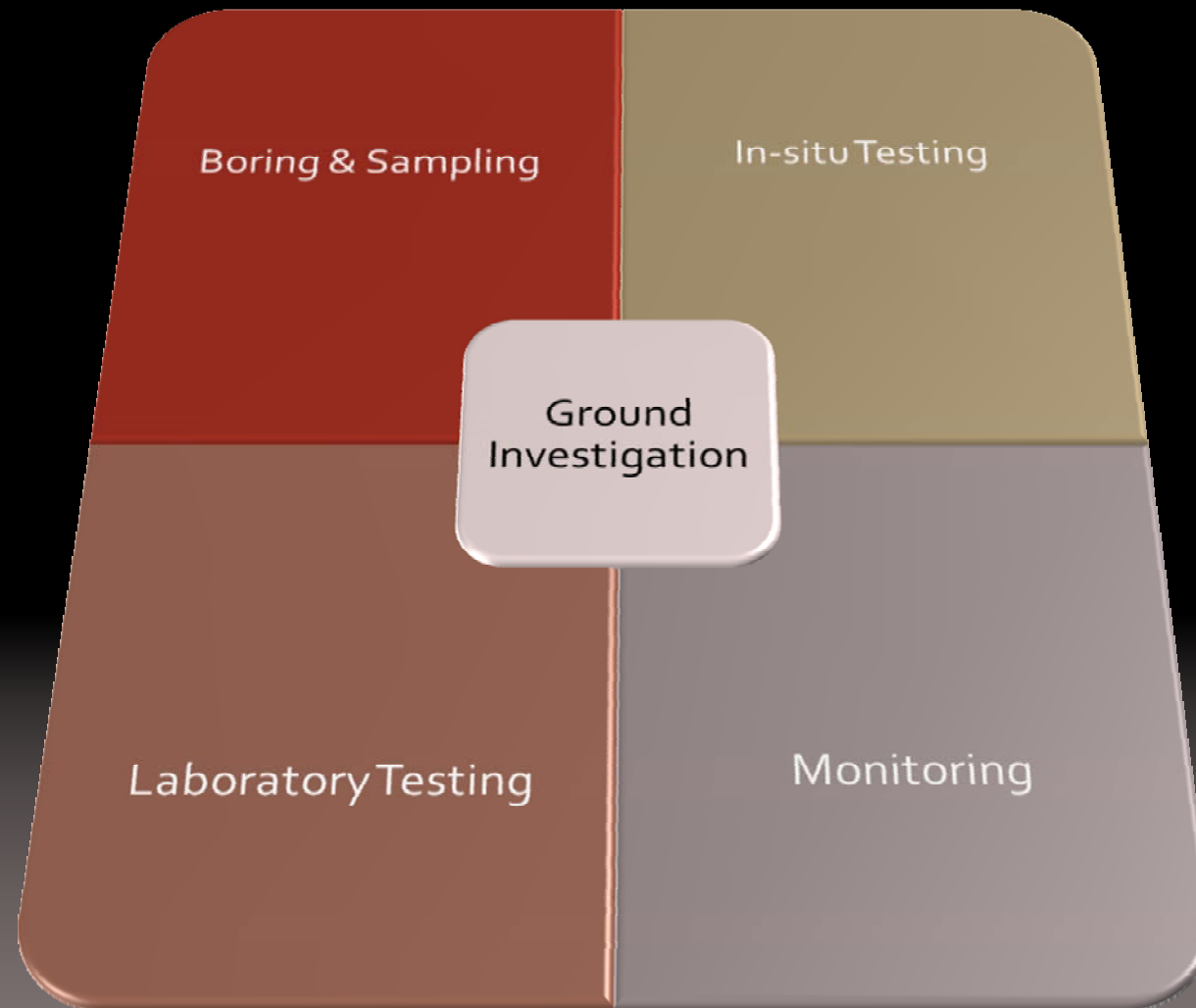


Consequence

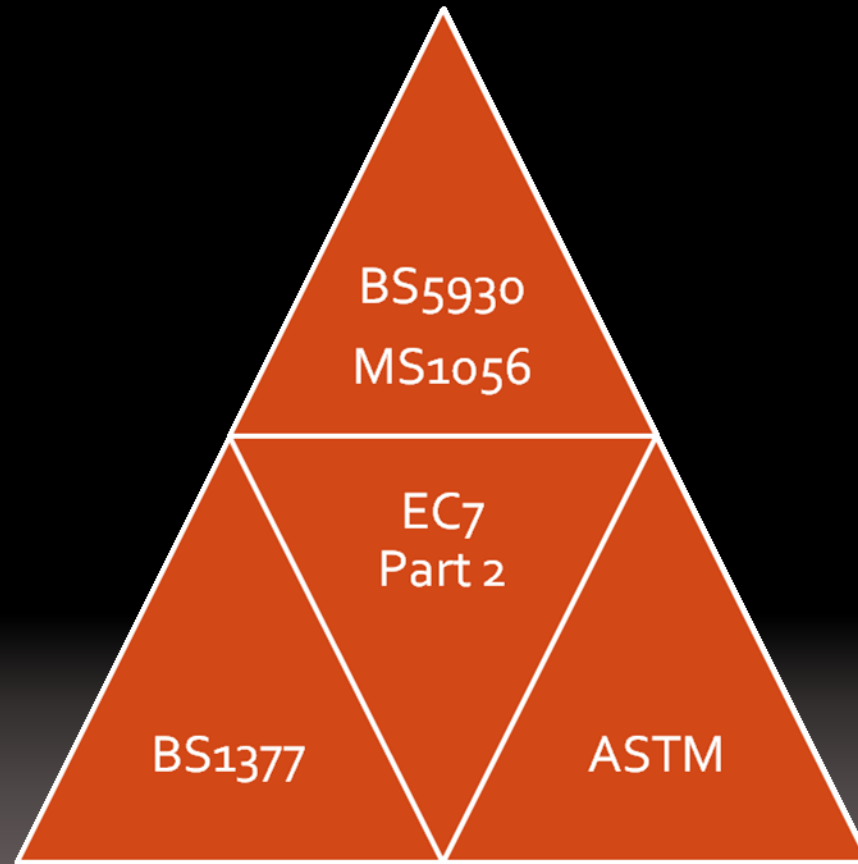


How GI shall be done ?

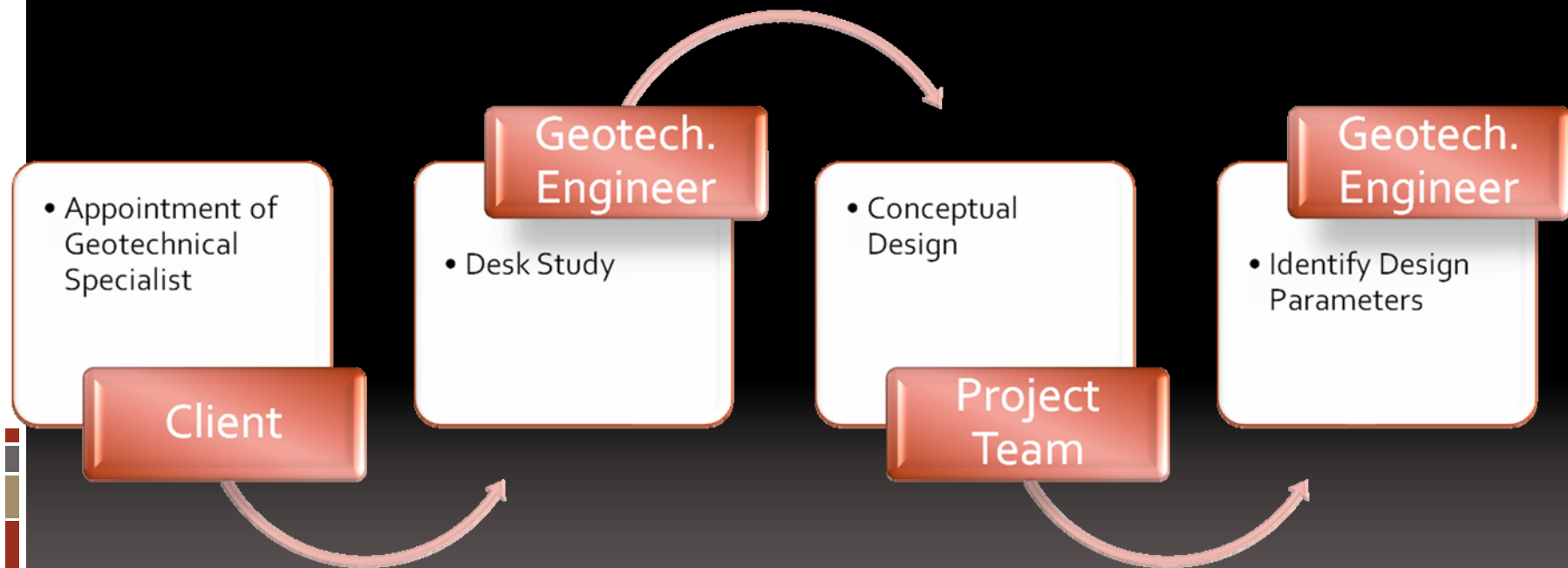




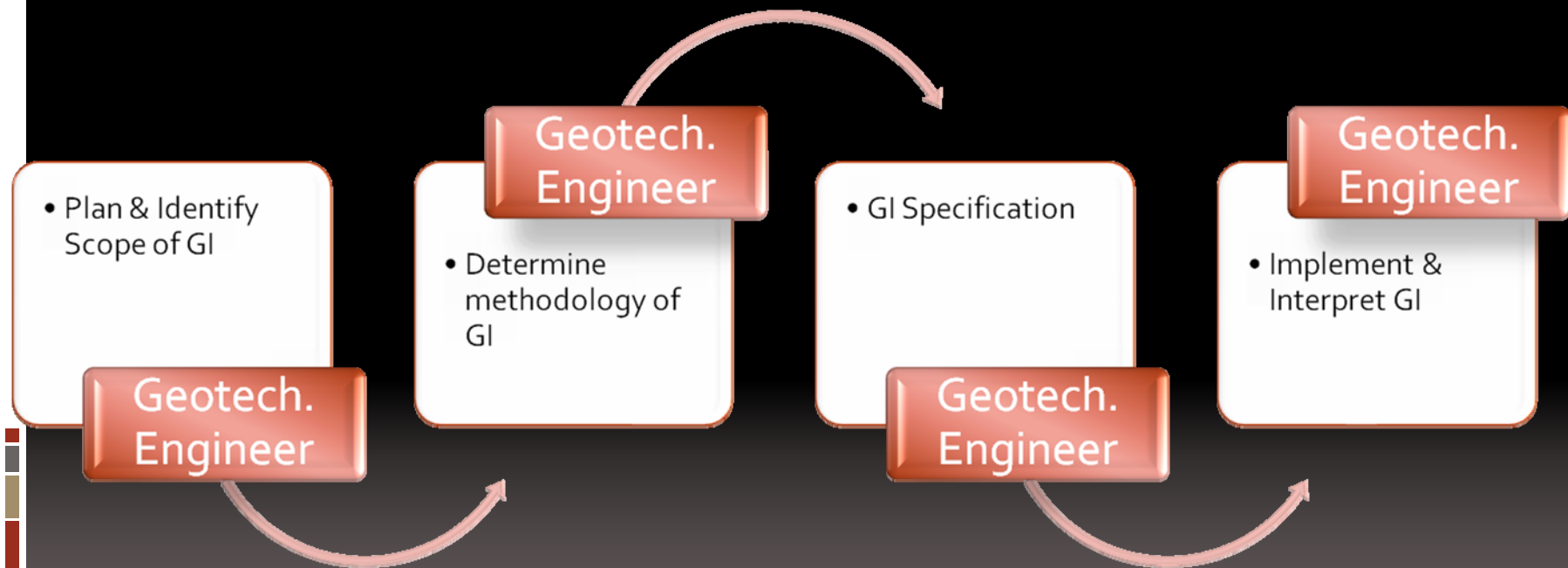
Codes & Standards



Process Diagram of Ground Investigation



Process Diagram of Ground Investigation



Stage 1 of GI



Desk Study



Site Walk-over Survey



Identify Project Need

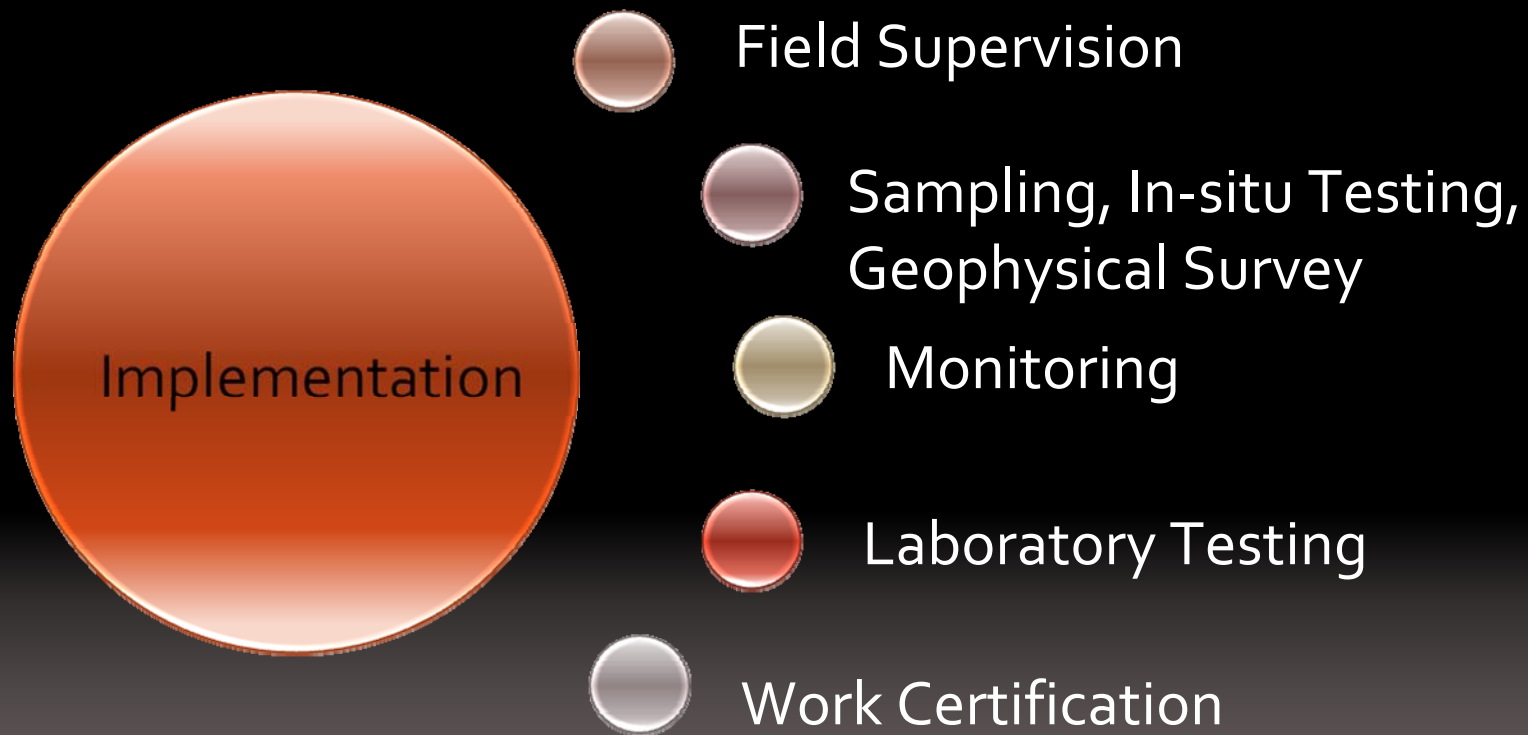


Scope of GI

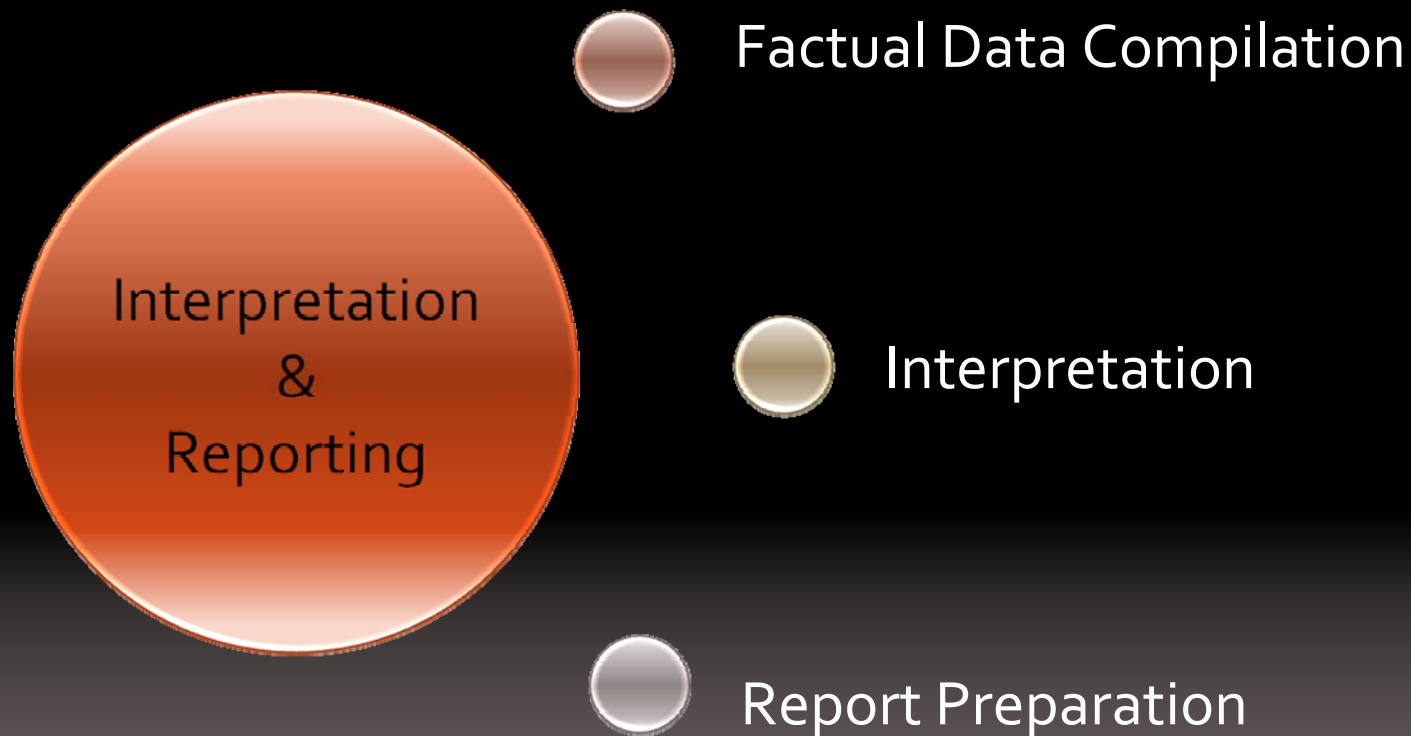


Bid Document & Tender

Stage 2 of GI



Stage 3 of GI

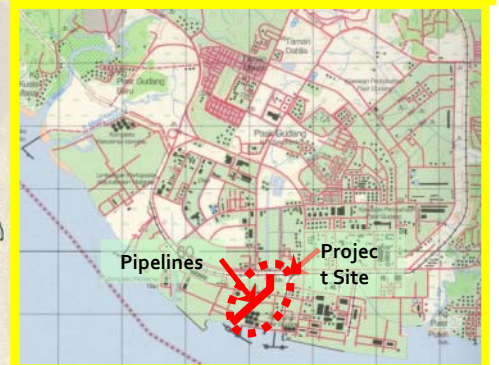
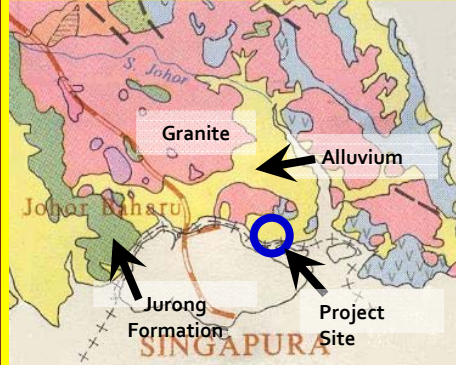
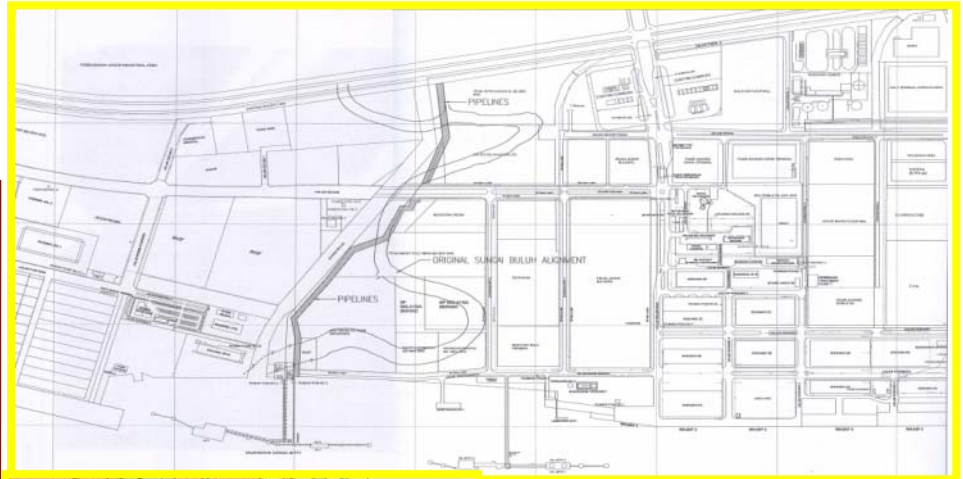
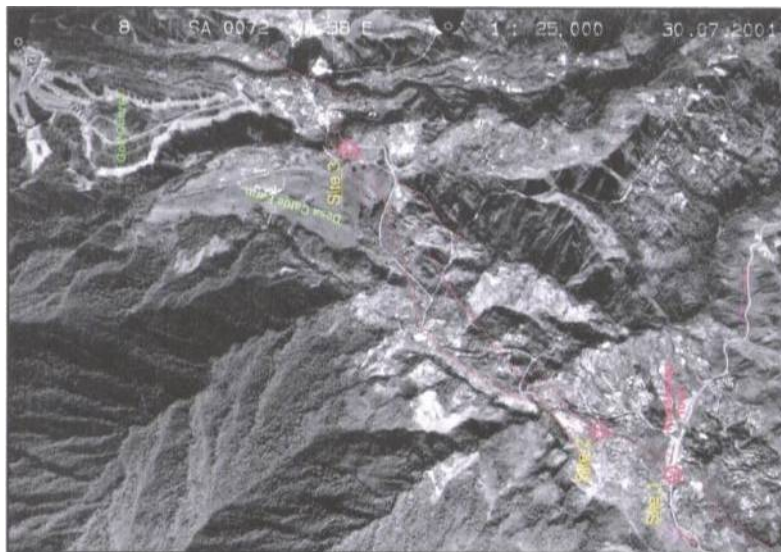


“Without Site Investigation, Ground is a Hazard”

Desk Study

Information for Desk Study :

- Topographic Maps
- Geological Maps & Memoirs
- Site Histories & Land Use
- Aerial Photographs
- Details of Adjacent Structures & Foundation
- Adjacent & Nearby Ground Investigation



Site Walkover Survey

- Confirm the findings from Desk Study
- Identify additional features & information not captured by Desk Study



GI Planning

Layout

- Direct influence beneath the proposed structure/works
- Distant Impact from the proposed structure/works

Frequency

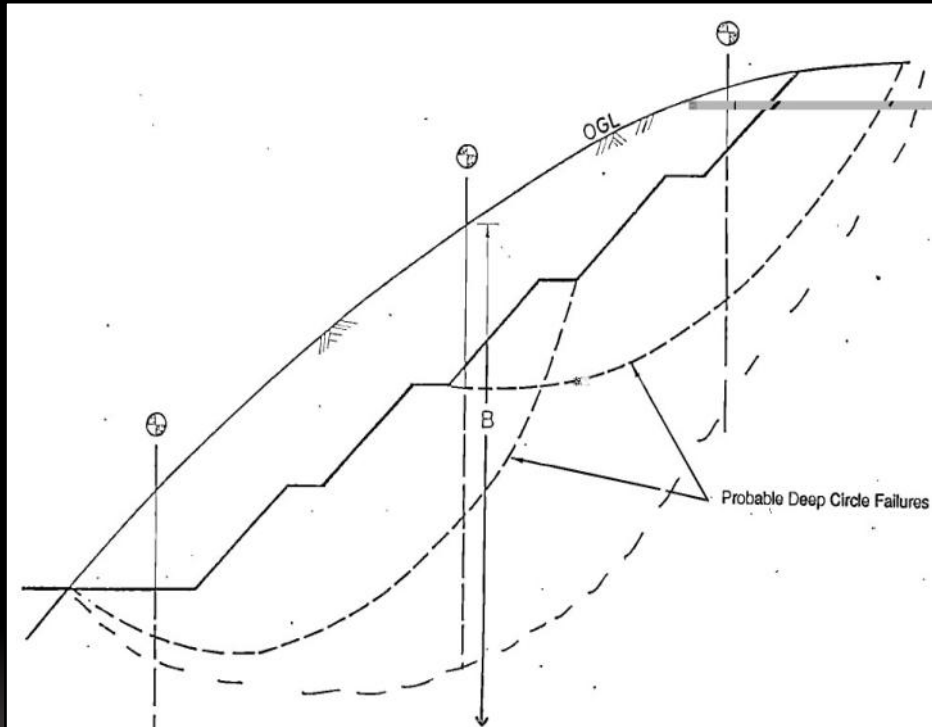
- Light structure
- Compact structure (3 ~ 5 points)
- Linear infrastructure (Representation of each geological unit)
- Slope : 3 probing per critical section

Vertical Extent

- Foundation : 10% stress bulb or to competent founding strata
- Slope : Hard strata or bedrock, not less than overall slope height

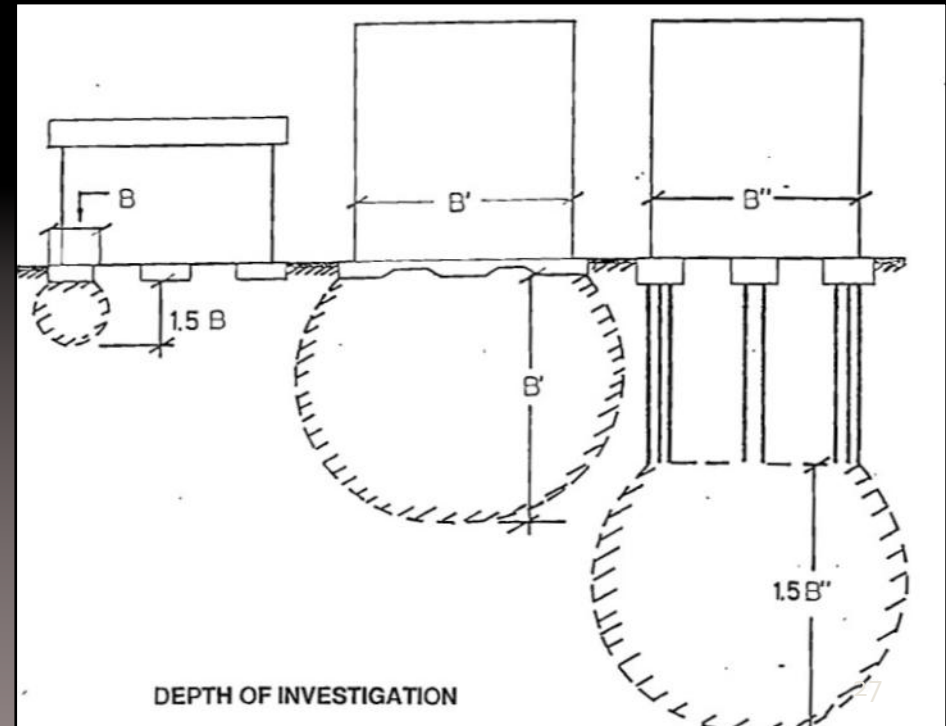
Watch-out for boulder, cavity, hard pan, necessary depth for weathering profile

Depth of Investigation



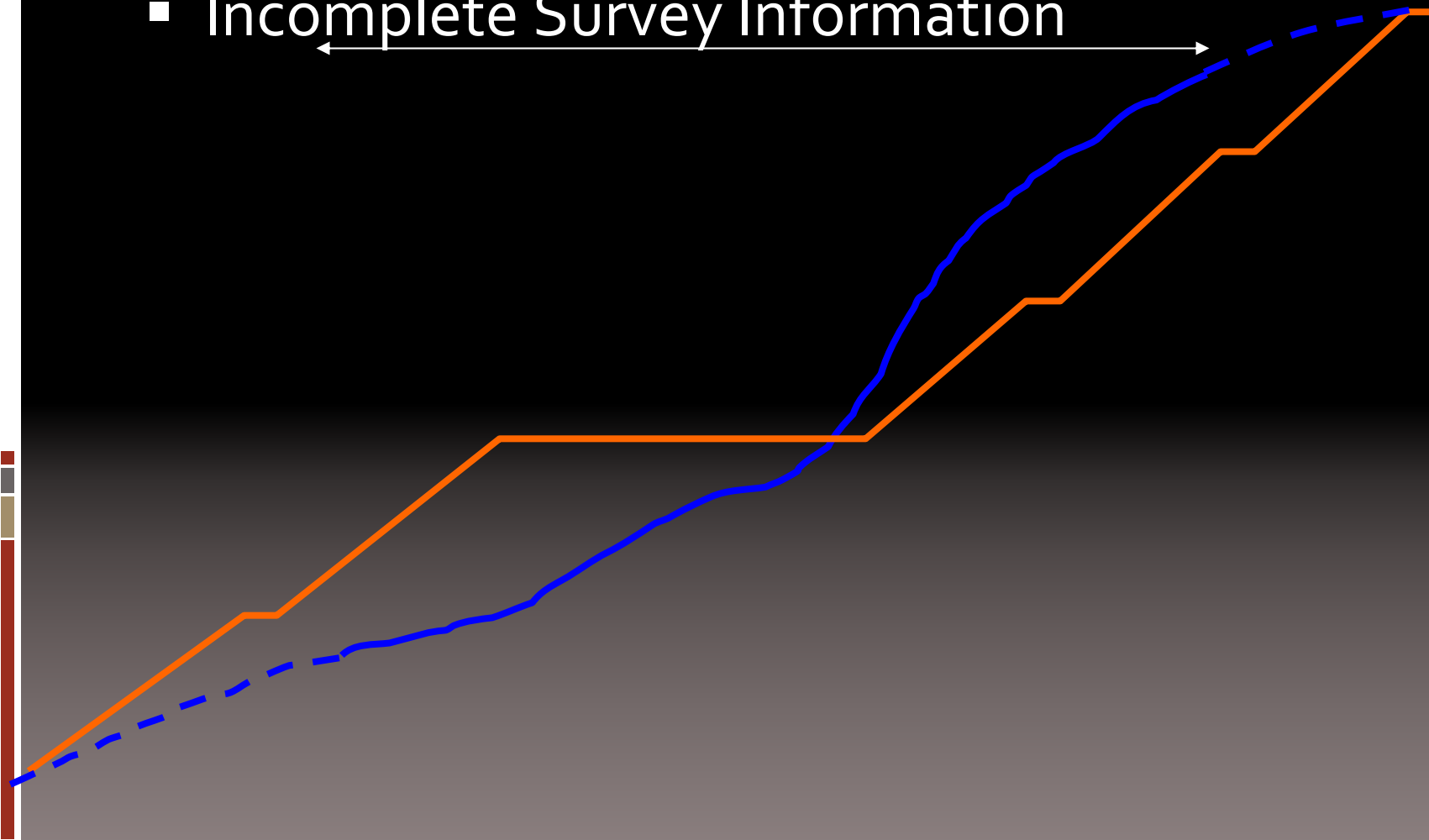
Stability Analysis

Foundation Design



Common Problems

- Incomplete Survey Information



GI Planning

Sampling

- Reasonable samples in each soil strata and bedrock
- Groundwater samples

In-situ Test

- Advance indication on strength, stiffness, permeability
- Direct testing
- Less sample size effect
- In-situ stress

Laboratory Test

- Sample quality & disturbance
- Late availability of result
- Stress path controlled & effective strength are possible under controlled environment

GI Planning

Monitoring

- Ground movement (sliding surface, settlement/compression)
- Groundwater Fluctuation
- Appropriate timing & monitoring duration
- Identify potential failure mechanism

Stage of Investigation

- Preferably in three stages (strategically)
 - Preliminary GI with contingency provision – Broad overview of ground conditions
 - Detailed GI – At critical areas for more information
 - GI in Construction Verification - Areas not covered in previous GI/design modification

Flexibility

- Allow for flexibility of information coverage catering for option exploration

Specification

- Objectives (study, design, forensic, construction)
- Type of investigation, mapping & field survey
- Vertical & lateral extent (termination depth)
- Sampling requirements (types, sampling locations & techniques)
- In-situ and laboratory testing requirements (standards)
- Measurement/monitoring requirements (instrument types & frequency)
- Skill level requirements in specialist works & interpretation
- Report format & data presentation



Specification

- Work schedule & GI resources planning
- Payments for services, liability, indemnity, insurance cover

Boring/Drilling

Recover Sample

- Subsurface stratification/profile
- Material classification & variability
- Laboratory tests

In-situ Testing

- Allow in-situ tests down hole (profiling)
- Direct measurement of ground behaviours

Monitoring

- Allow monitoring instruments installed down hole

Direct Method – Boring, Sampling, In-situ & Laboratory Testing

Medical Applications

- Biopsy sampling

Geotechnical Applications

- Boring, Trial Pitting & Sampling

- Thin-walled, Piston Sampler
- Mazier Sampler
- Block Sample

- In-situ Testing

- SPT, MP, CPT_u, VST, PMT, DMT, PLT,
- Permeability Test
- Field Density Test

- Laboratory Testing

- Classification Test
- Compressibility Test (Oedometer/Swell)
- Strength Test (UU/UCT/CIU/DS)
- Permeability Test
- Compaction Test
- Chemical Test (pH, Cl, SO₄, Organic Content, Redox, etc)
- Petrography & XRD



Indirect Method – Geophysical Survey

Medical Applications

- X-ray, Computer Tomography & MRI
- Ultra-sound

Geotechnical Applications

Geophysical Survey

- Electromagnetic Waves
(*Permeability, Conductivity & Permittivity*)
- Mechanical Wave
(*Attenuation, S-waves & P-waves*)
 - Resistivity Method
 - Microgravity Method
 - Transient Electro-Magnetic Method
 - Ground Penetration Radar
 - Seismic Method

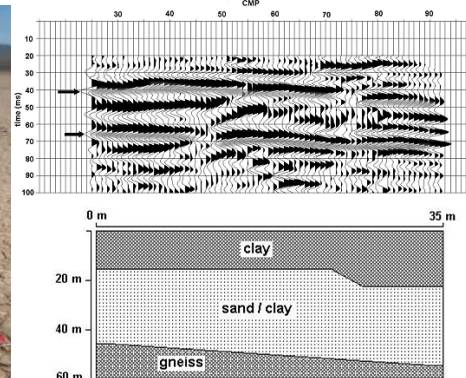
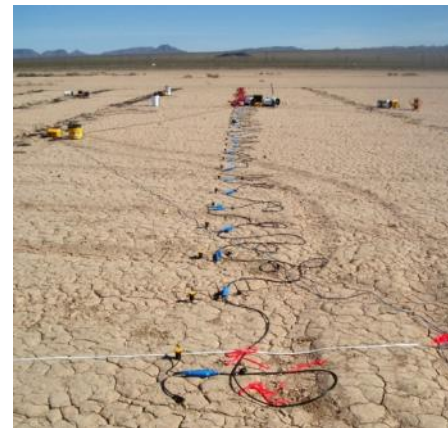
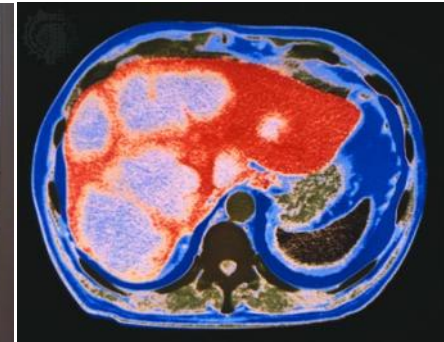
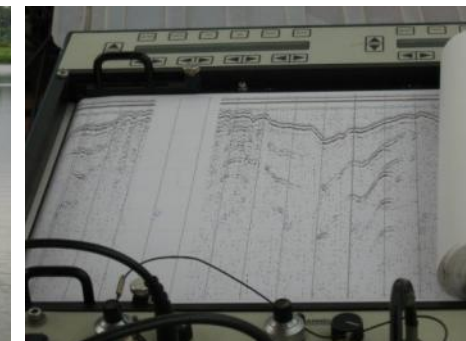
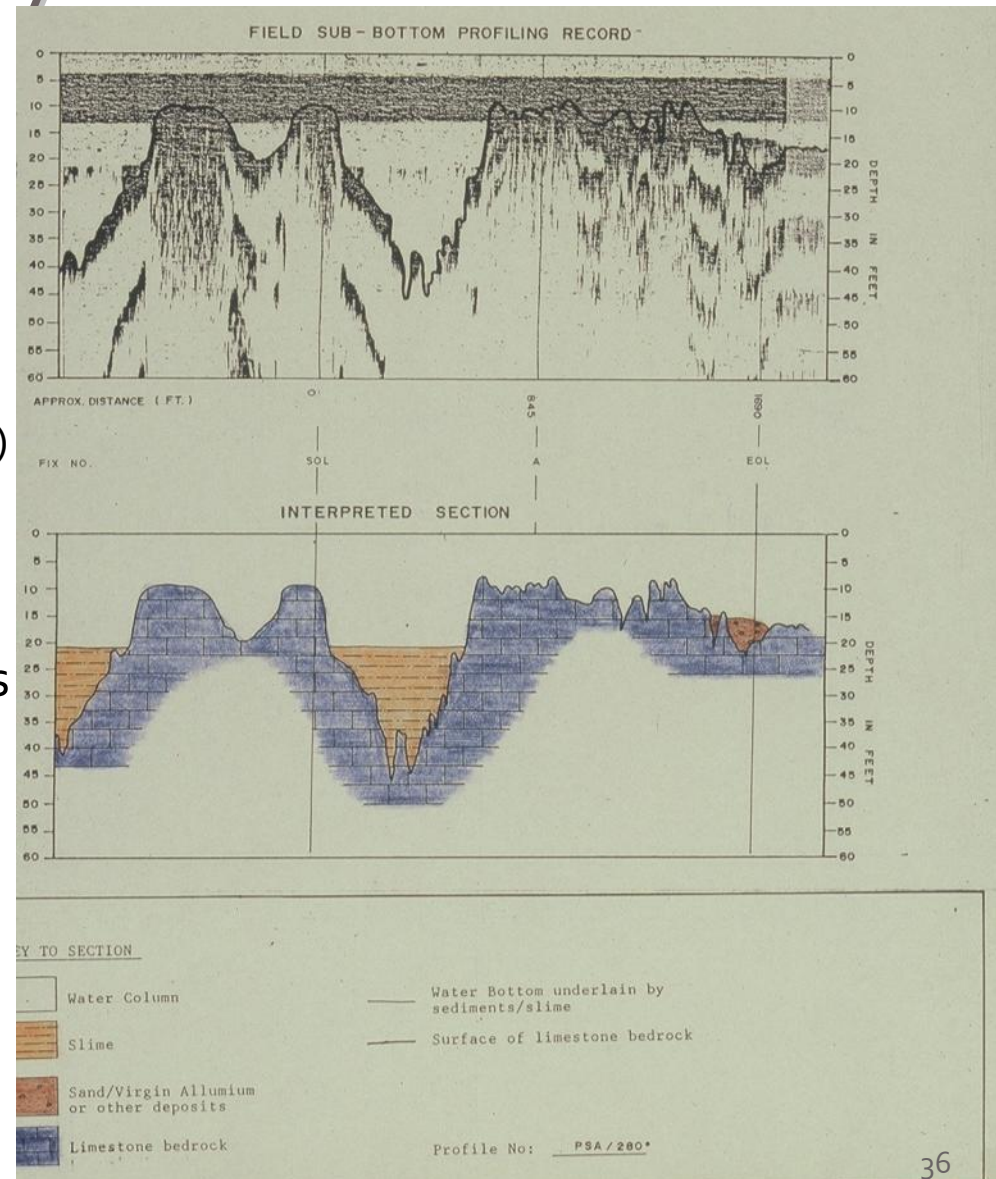


Figure 9 – Final stacked section of the Area 2.
Figura 9 – Seção sísmica empilhada final e seção interpretada da Área 2.



Geophysical Survey

- Merits
 - Lateral variability (probing location)
 - Profiling (sampling & testing)
 - Sectioning (void detection)
 - Material classification
 - Engineering parameters (G_o & $G_{dynamic}$)
- Problems
 - Over sale/expectation
 - Misunderstanding between engineers, engineering geologists & geophysicists
 - Lack of communication
 - Wrong geophysical technique used
 - Interference/noise



Sampler

Split Spoon

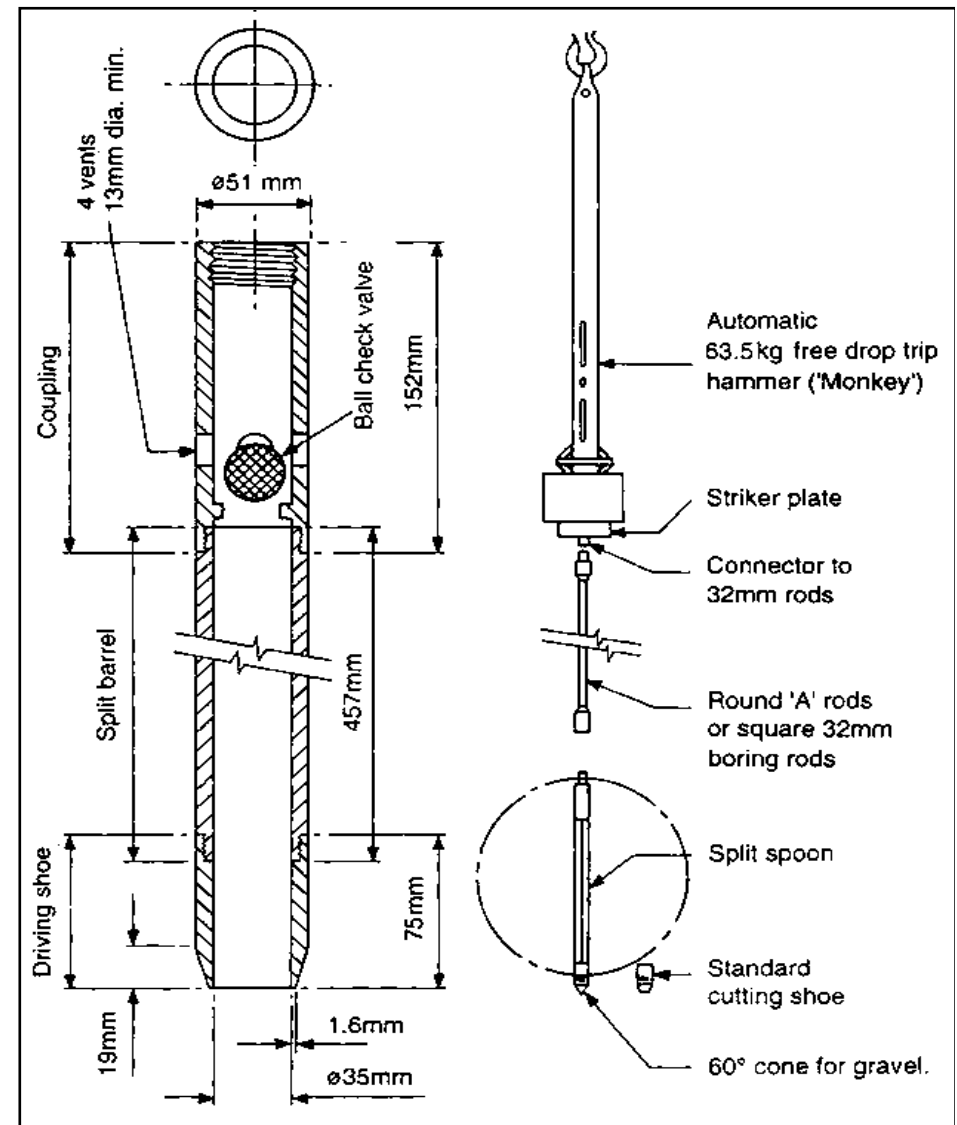
Thin-Walled

Piston Sampler

Mazier Sampler

Core Barrel

Wire-line



Sampler

Split Spoon

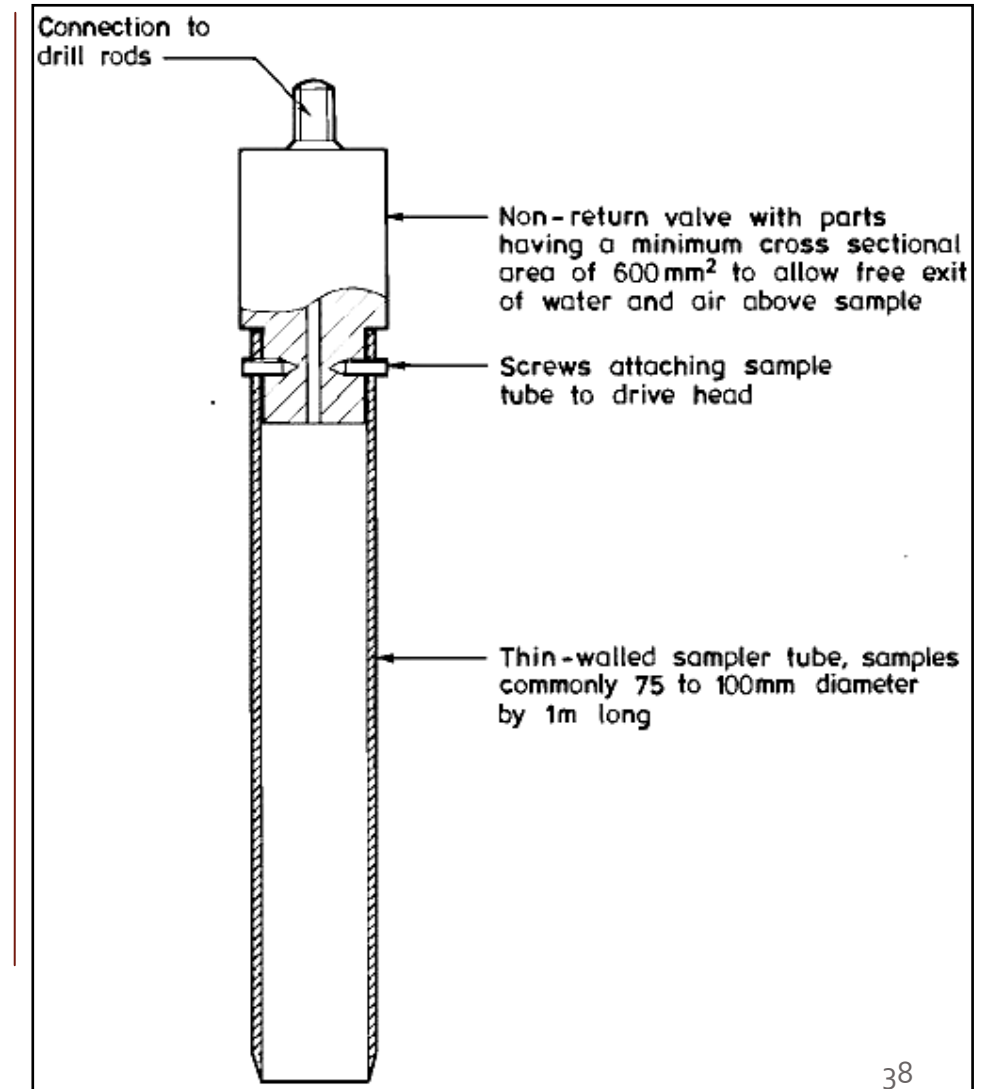
Thin-Walled

Piston Sampler

Mazier Sampler

Core Barrel

Wire-line



Sampler

Split Spoon

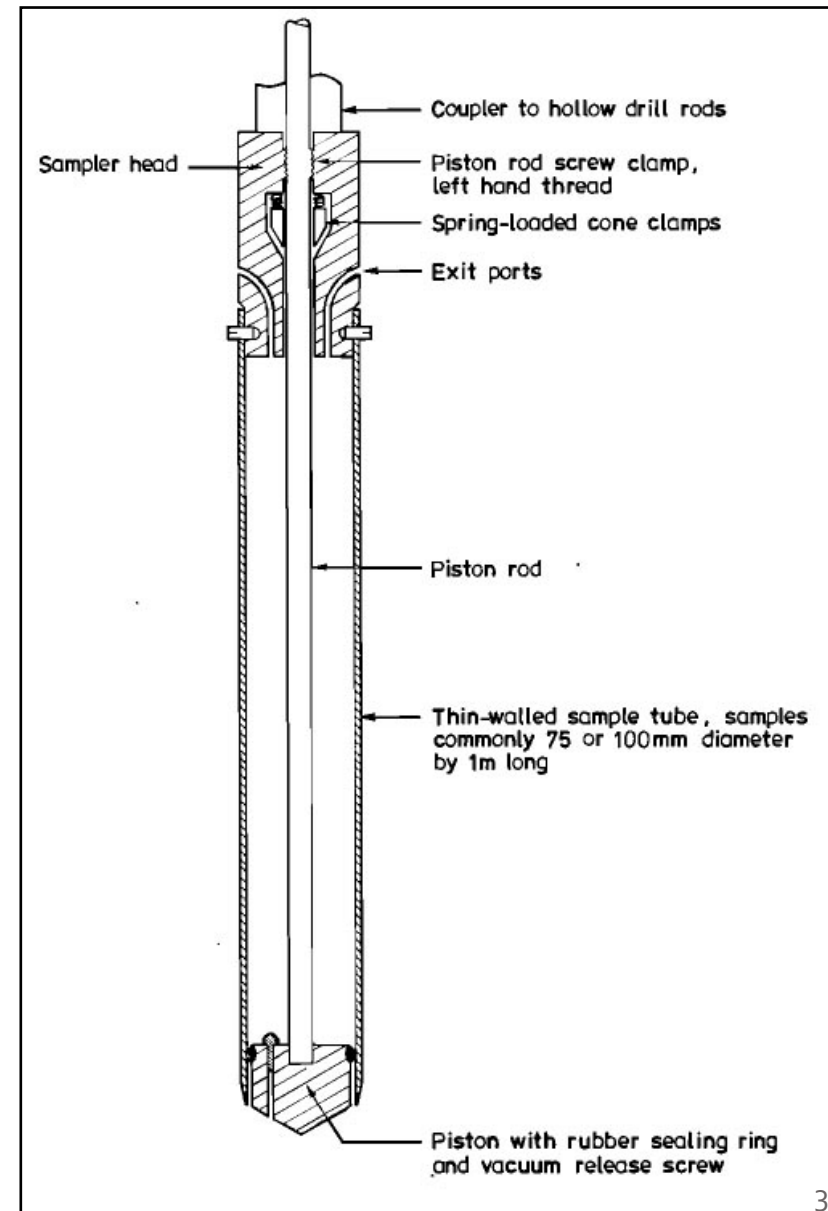
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Piston Sampler

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Wire-line



Sampler

Split Spoon

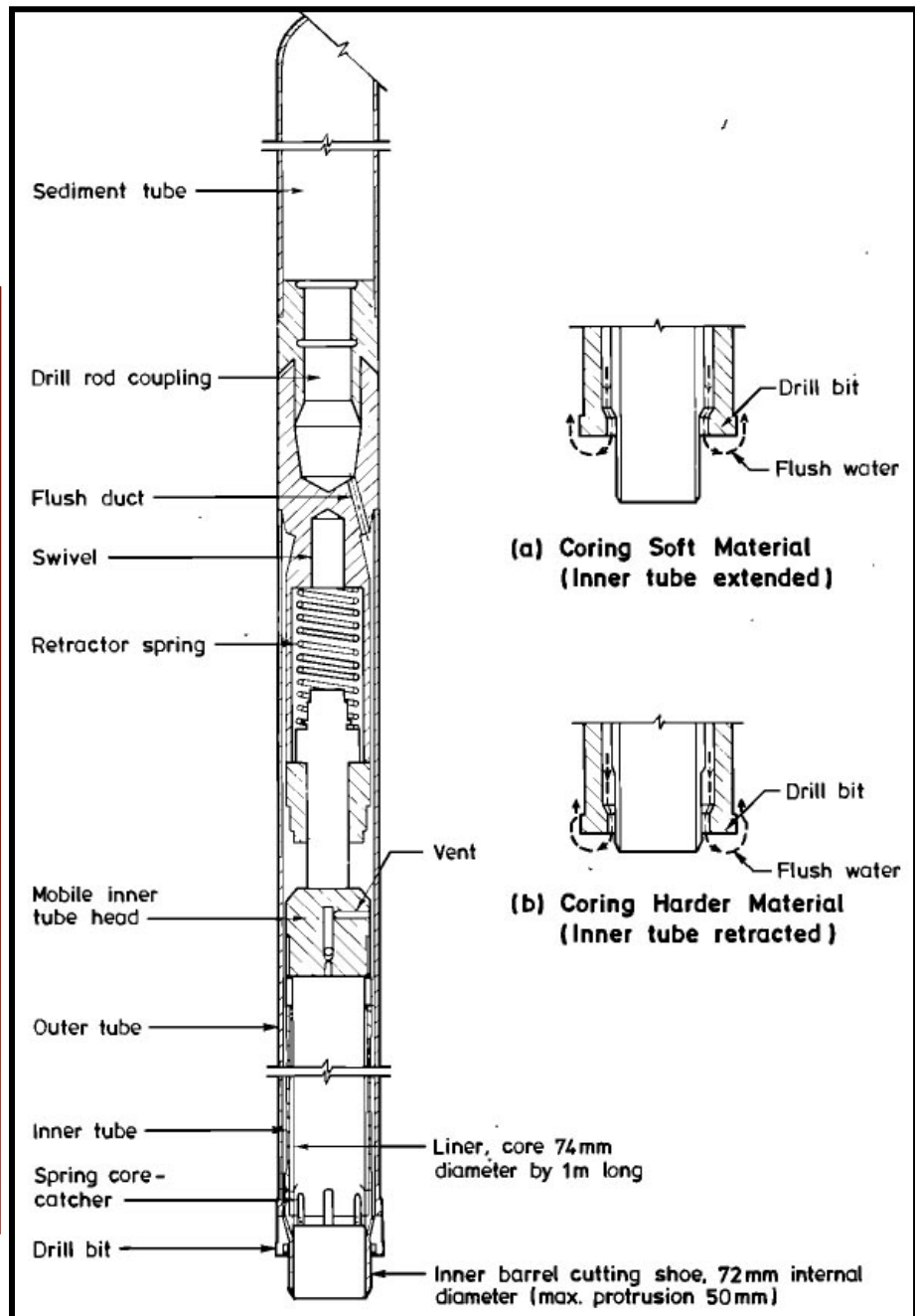
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Piston Sampler

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Sampler

Split Spoon

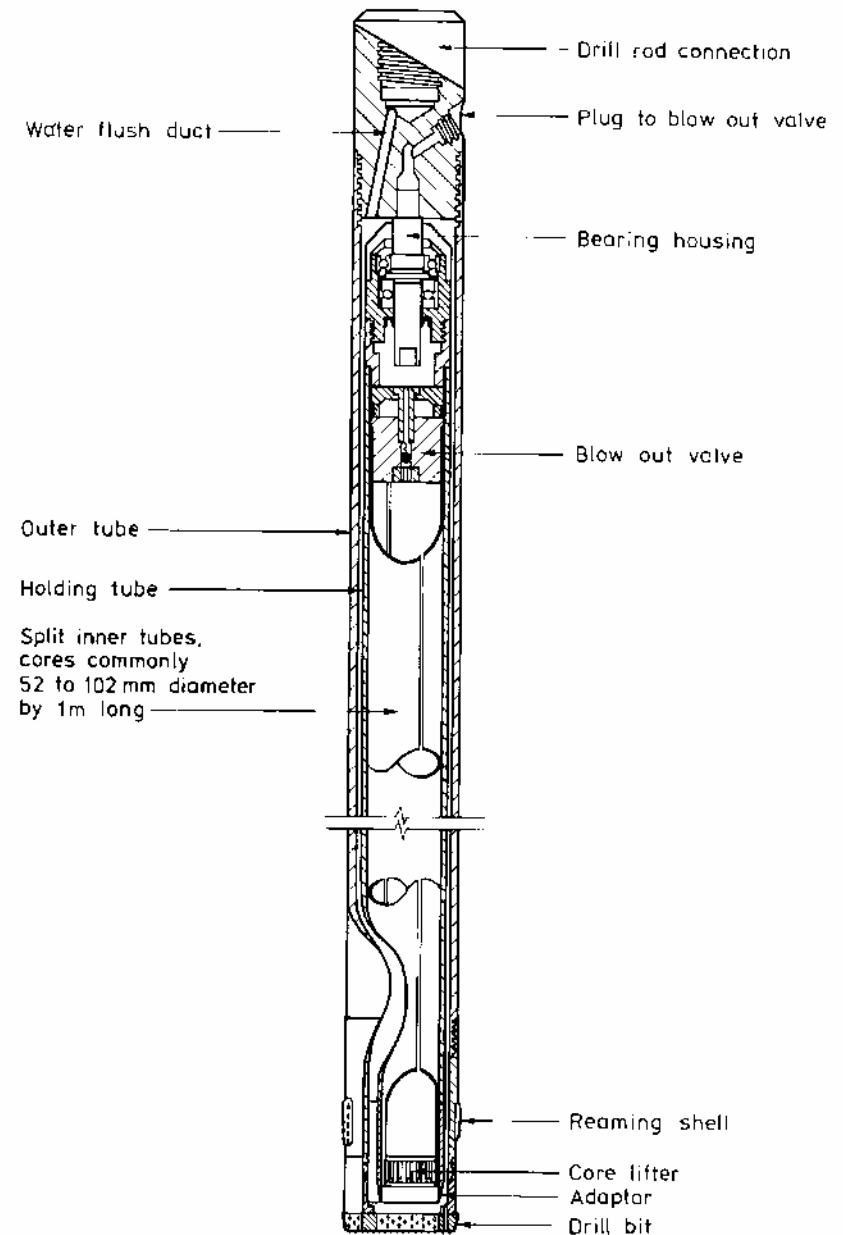
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Sampler

Split Spoon

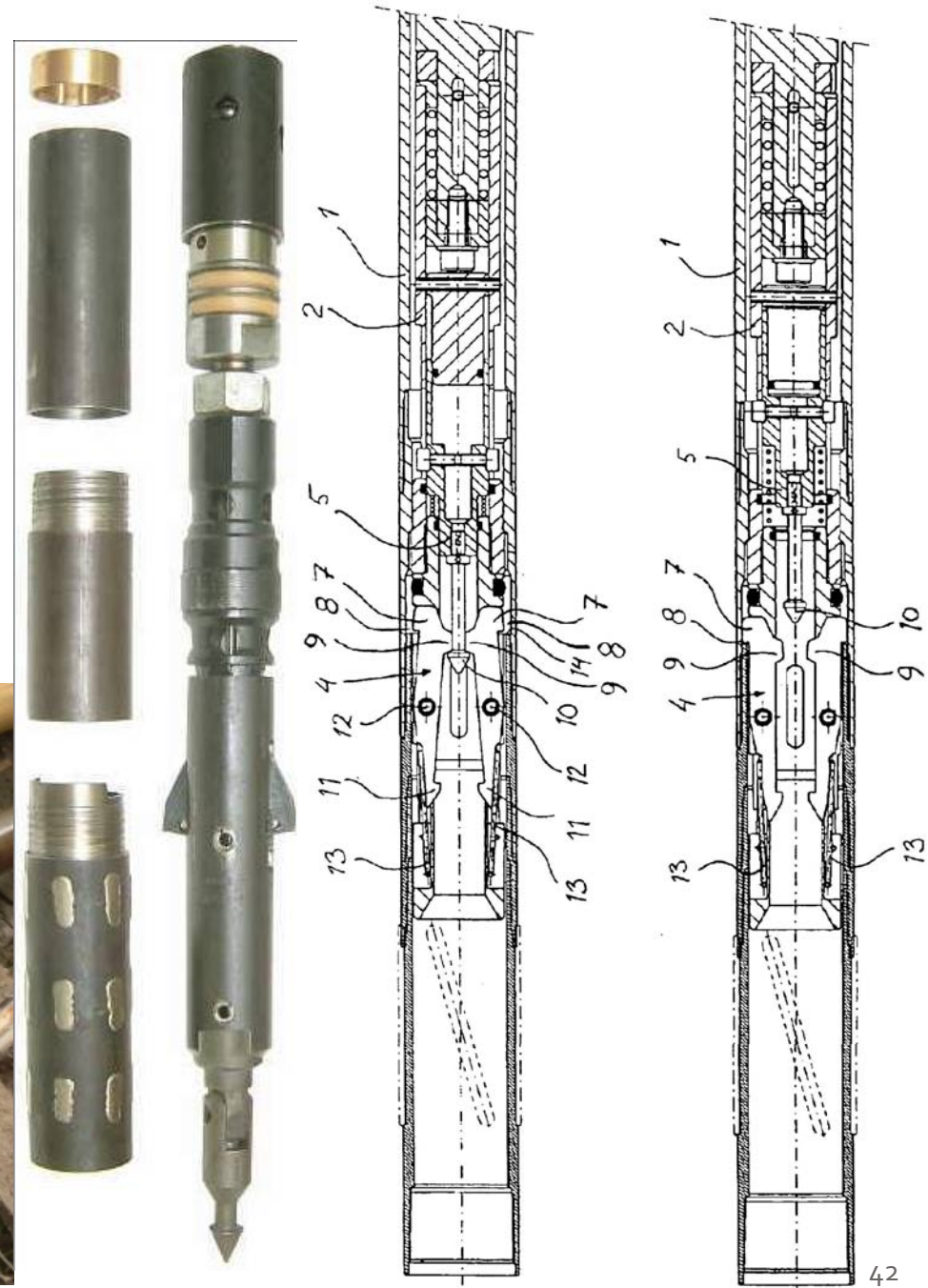
Thin-Walled

Piston Sampler

Mazier Sampler

Core Barrel

Wire-line



Sample Storage, Handling, Transportation



Sample Preparation



Sampling

- Sample Sizes

- Representative mass (particle sizes, fabric, fissures, joints)
- Adequate quantity for testing

- Sample Disturbance

- Stress conditions
- Deformation behaviours
- Moisture content & void
- Chemical characteristics

At Different Stages of SI

Before	During	After
Stress relief	Stress relief	Stress relief
Swelling	Remoulding	Moisture migration
Compaction	Displacement	Extrusion
Displacement	Shattering	Moisture loss
Base heave	Stone at cutting shoe	Heating
Piping	Mixing or segregation	Vibration
Caving	Poor recovery	Contamination

Clayton et al (1982)

Sample Disturbance

- Poor recovery
 - Longer rest period for sample swelling
 - Slight over-sampling
 - Use of sample retainer
- Sample contamination



Sample Quality Classification

Sample Quality	Soil Properties					
	Classification	Moisture Content	Density	Strength	Deformation	Consolidation
Class 1	✓	✓	✓	✓	✓	✓
Class 2	✓	✓	✓	✗	✗	✗
Class 3	✓	✓	✗	✗	✗	✗
Class 4	✓	✗	✗	✗	✗	✗
Class 5	✗	✗	✗	✗	✗	✗

BS 5930 (1981)

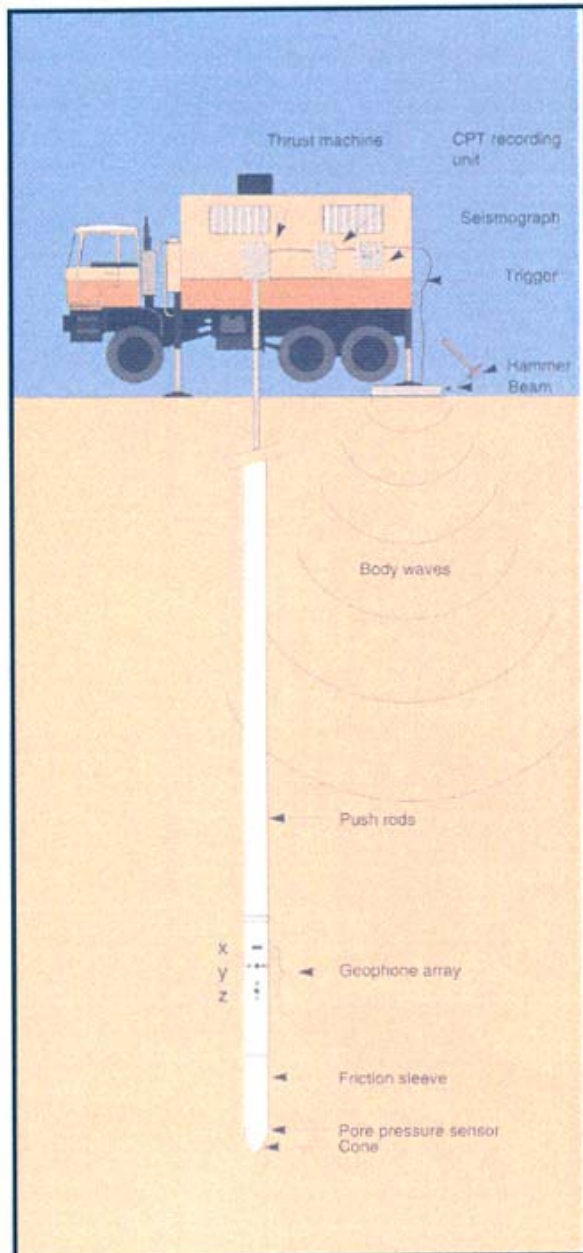
|| In-situ Tests



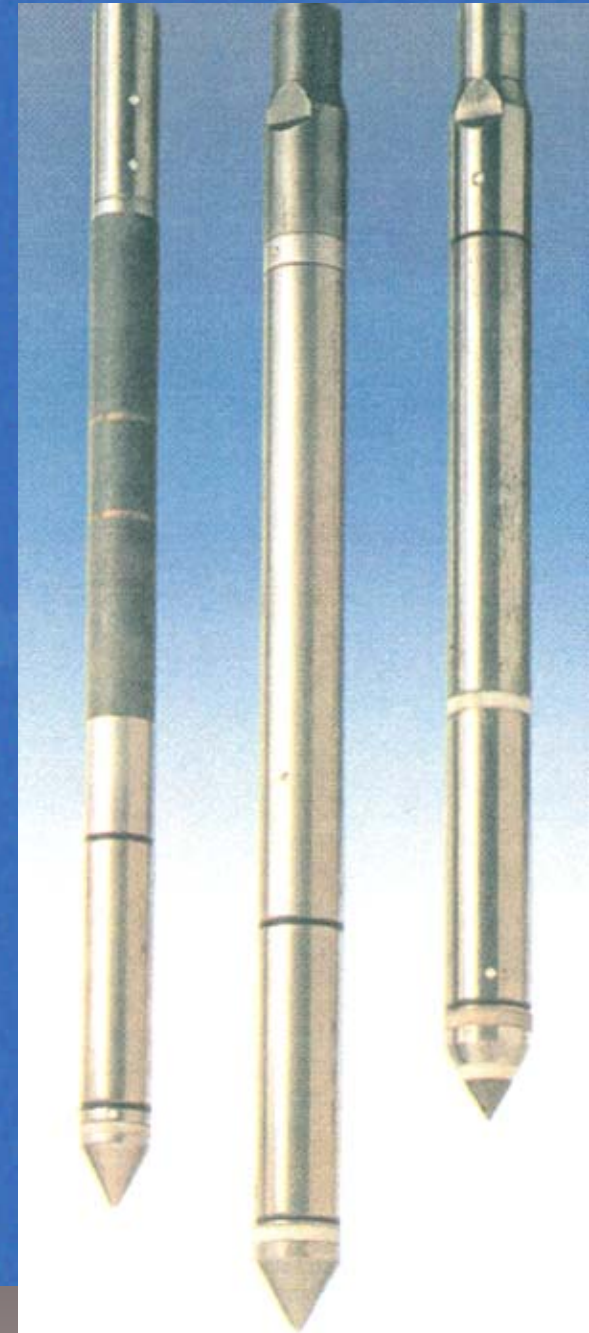
Piezocone (CPTu)



CPT Equipment



- The Seismic Cone Penetration Test (SCPT) is a reliable, cost effective technique to determine the insitu seismic wave velocity.
- Seismic wave velocities give an indication of ground characteristics such as low strain shear modulus and Poisson's ratio.
- Data from the cone penetrometer is used in delineating the strata changes identified by the seismic results.



■ In-Situ Tests

- BS1377 : Part 9
- Suitable for materials with difficulty in sampling
 - Very soft & sensitive clay
 - Sandy & Gravelly soils
 - Weak & Fissured soils
 - Fractured rocks
- Interpretation
 - Empirical
 - Semi-empirical
 - Analytical

■ Applicability of In-situ Tests

Test	Stress	Strength			Stiffness			Permeability
	K_o	ϕ'	C_u	σ_c	E'/G	E_u	G_{max}	k
SPT		G	C	R	G	C	G	
CPT/CPT _u		G	C		G			
DMT	G, C				G			
Borehole PMT			C		G, R	C		
PLT			C		G, R	C		
VST			C					
Seismic							G, C, R	
SBPMT	G, C	G	C		G, C			
Falling/ Rising Head Test								G
Constant Head								C
Packer Test								R

Clayton , et al (1995)

G = granular, C = cohesive, R = Rock

■ Applicability of In-situ Tests

SUMMARY ON THE COMMON TYPES OF GROUND INVESTIGATION, FIELD TESTS, SAMPLING & LABORATORY TESTS

Description	Types of Ground Investigation						Field Test					Laboratory Test								
	JP	HA	TP	BH	PZ	GS	SPT	PM	PLT	VS	PW	C	M/C	γ	Con	UU	UCT	CIU	Chem	
1) Soft ground treatment	m	m	m	y	y	m	-	-	-	y	m	y	y	y	y	y	y	m	m	
2) Shallow foundation																				
cohesive soil	y	m	m	y	-	-	y	m	y	m	-	y	y	y	y	y	y	m	m	
non cohesive soil	y	m	m	y	m	-	y	-	y	-	-	y	y	y	-	-	-	-	m	
3) Pile Foundation																				
Fill ground	m	m	m	y	m	m	y	m	-	m	-	y	y	y	y	y	y	-	m	
Cut ground	-	-	m	y	-	m	y	-	-	-	-	y	y	y	-	-	-	-	m	
4) Slope																				
Cut	-	-	m	y	-	y	y	-	-	-	y	y	y	y	-	y	y	y	-	
Fill	m	m	m	y	m	m	y	-	-	y	m	y	y	y	y	y	y	y	m	

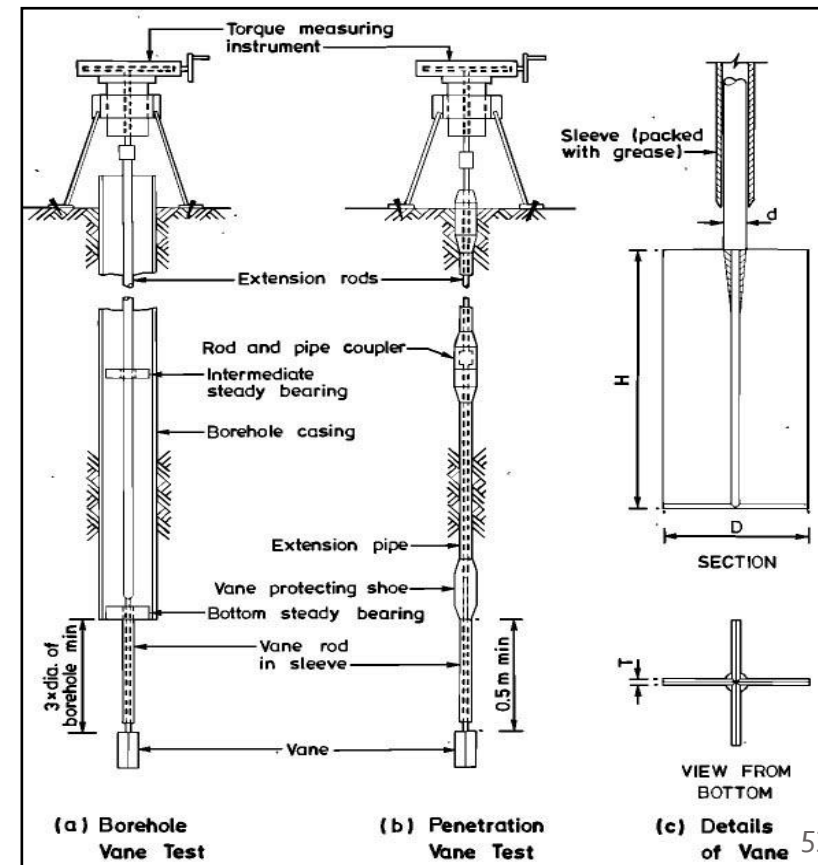
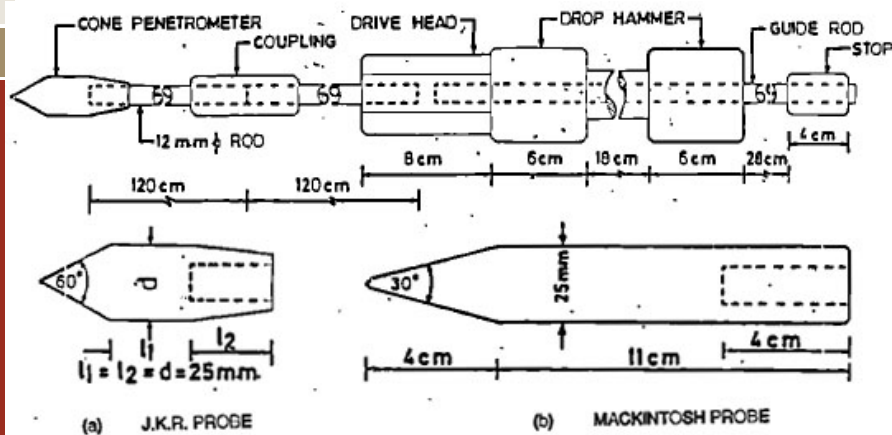
Legend:

JP : JKR Probe
 HA : Hand Auger
 TP : Trial Pit
 BH : Borehole
 PZ : Piezocone
 GS : Geophysical Survey

SPT : Standard Penetration Test
 PM : Pressuremeter
 PLT : Plated Bearing Test
 VS : Vane Shear Test
 K : Permeability Test
 Y : Yes should be done
 M : May be added
 - : Not relevant/necessary

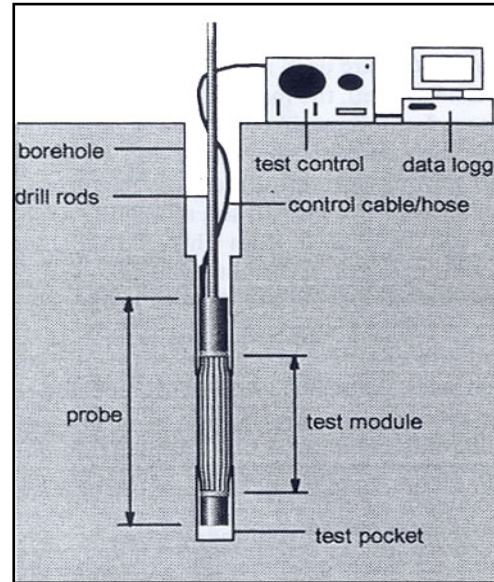
C : Classification
 M/C : Moisture Content
 γ : Unit Weight
 Con : Consolidation
 UU : Unconsolidated Undrained
 UCT : Unconfined Compression
 CIU : Triaxial with Pore Water Pressure
 Chem : Chemical Test

In-situ Tests



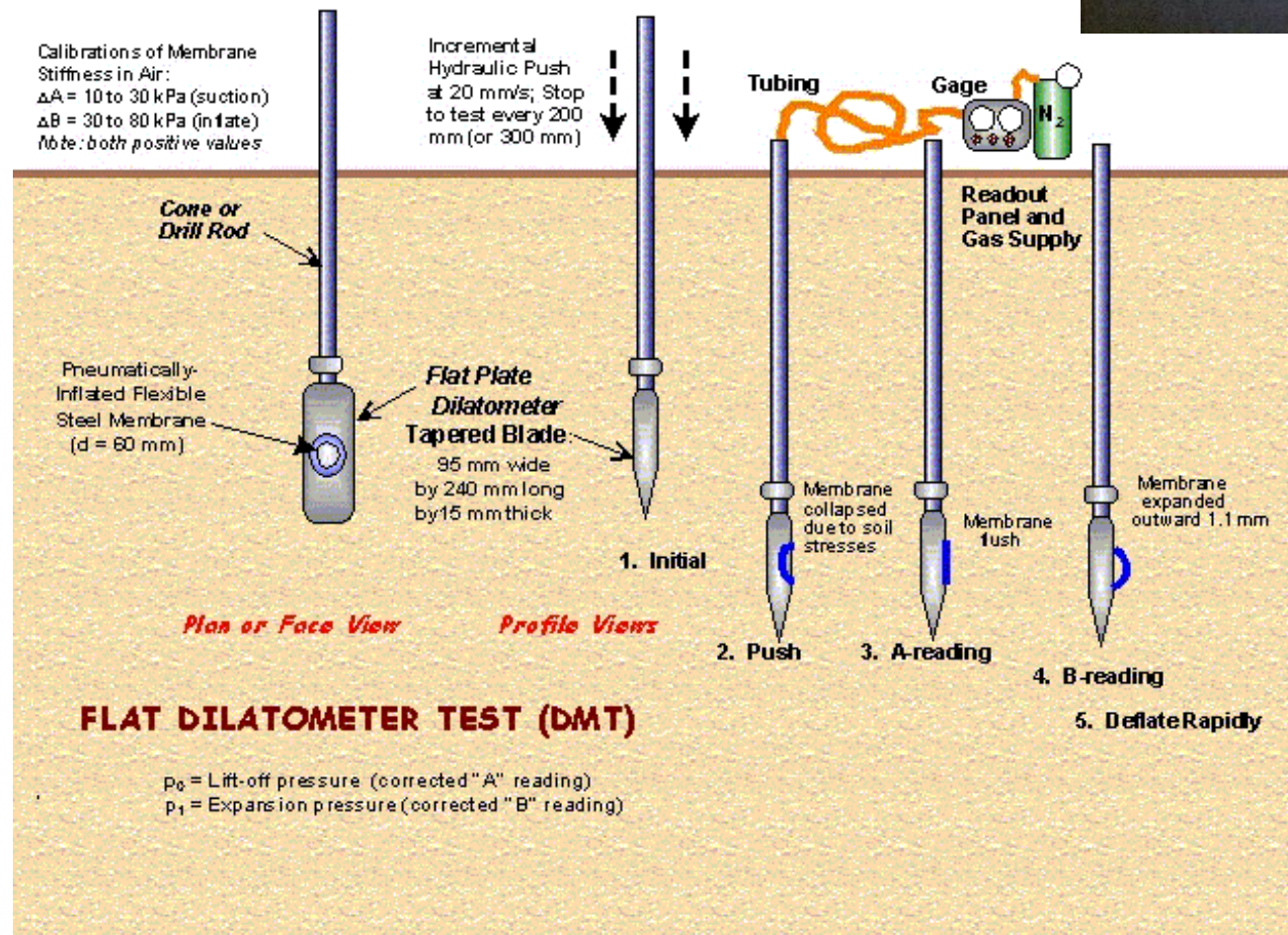
|| In-situ Tests

Pressuremeter (PMT)



In-situ Tests

Dilatometer (DMT)

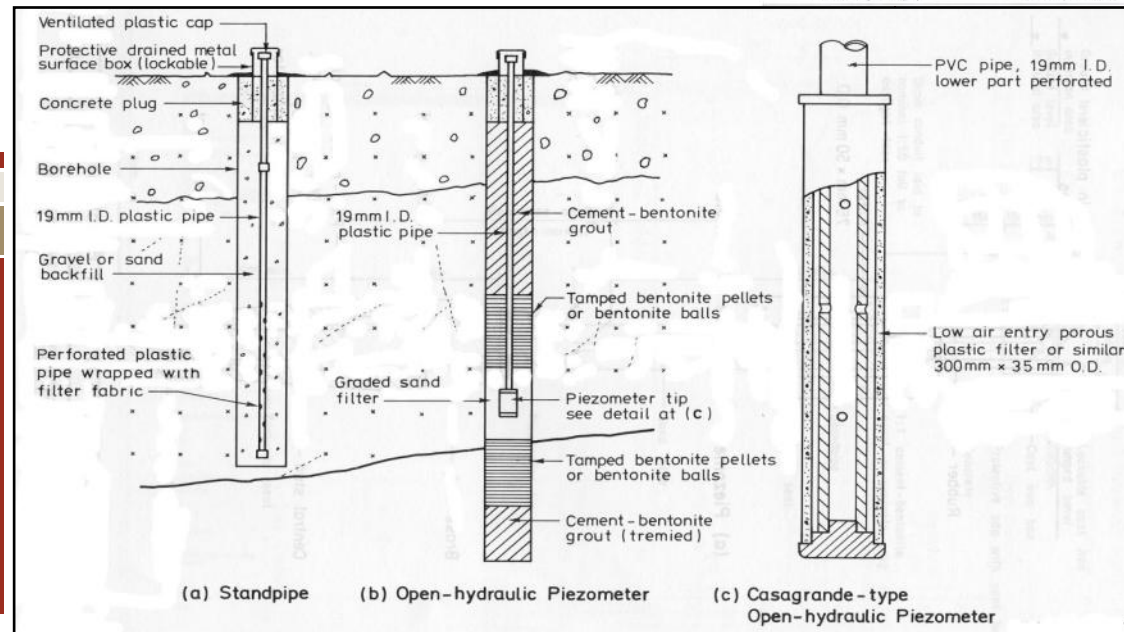
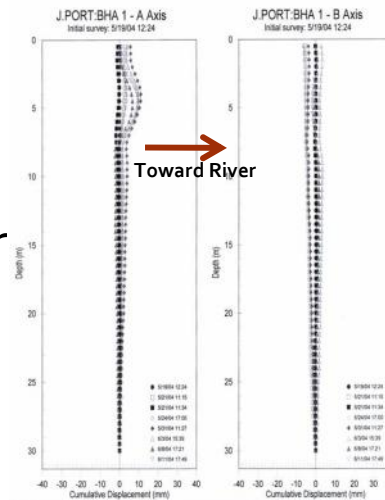


|| Geophysics Methods



Instrumentation Monitoring

- Inclinometer
- Extensometer
- Rod Settlement Gauge/Marker
- Piezometer
- Observation Well



Laboratory Tests



Table Top Geotechnical Centrifuge

e



4000 kg Compact Geotechnical Centrifuge



Vertical orientation for high speed running

A World First - The G-max Modular CgC

Soil Boxes compartmentalise the ring channel for small soil sample testing

Ring Channel for large soil sample testing at high 'g'

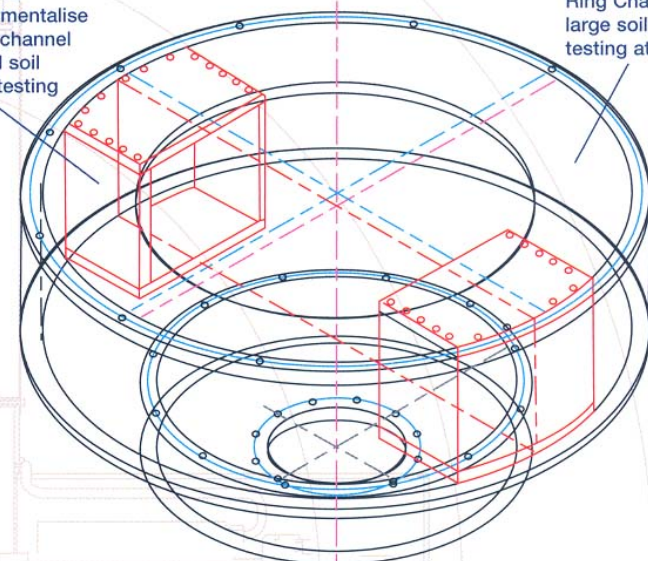


Fig 1: Drum Ring Channel with 2 Soil Compartment Boxes

Swing Platforms shown in position at centrifuge rest

Beam Centrifuge operation with effective payload radius 0.5m

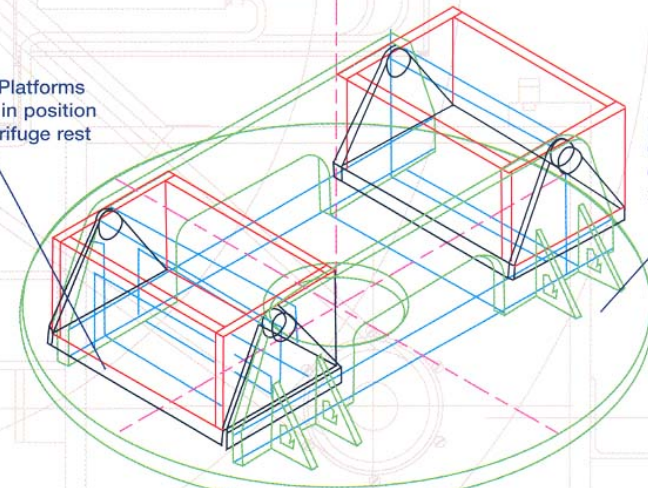
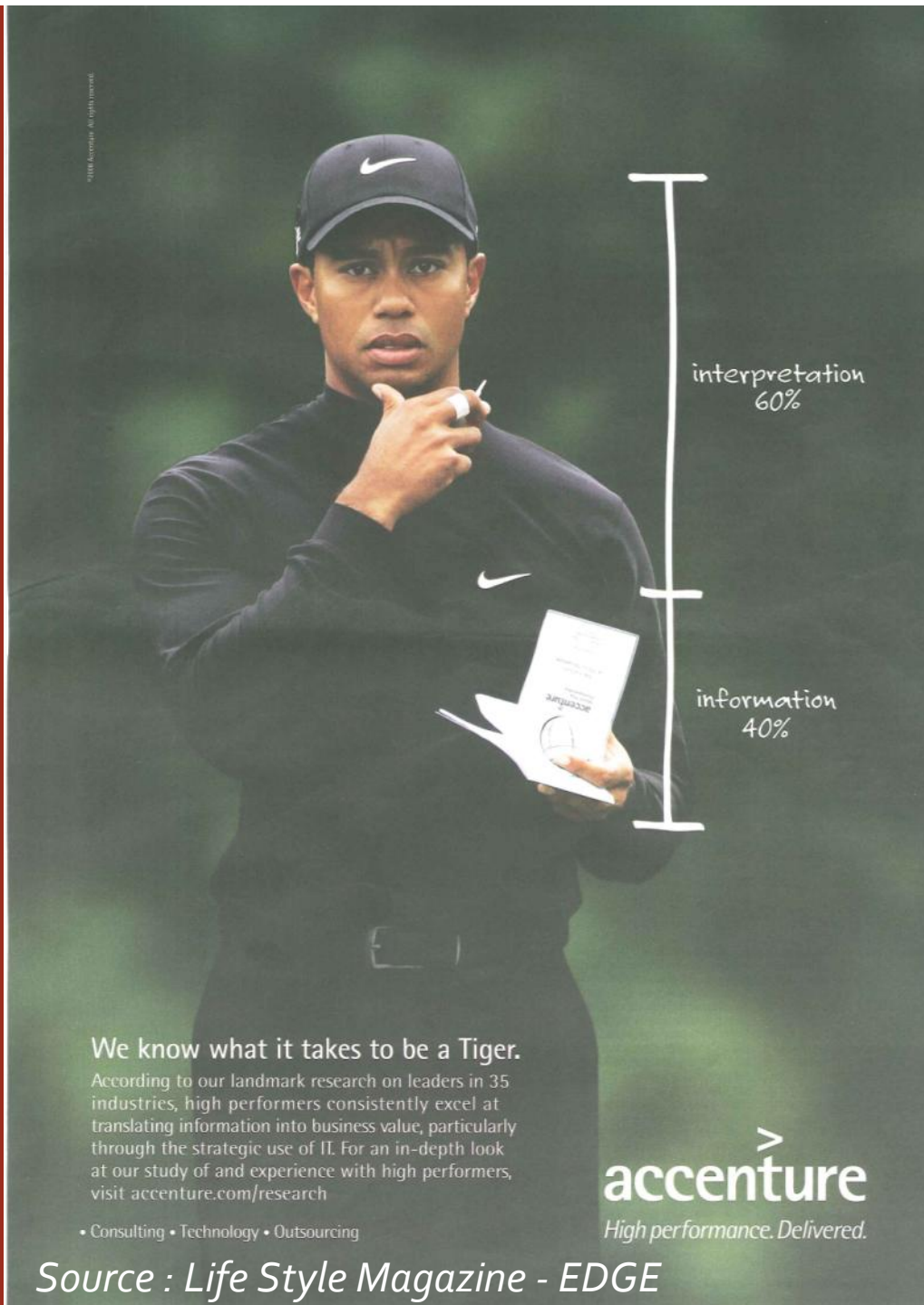


Fig 2: Beam Rotor Table with 2 Swinging Soil Strong Boxes



interpretation
60%

information
40%

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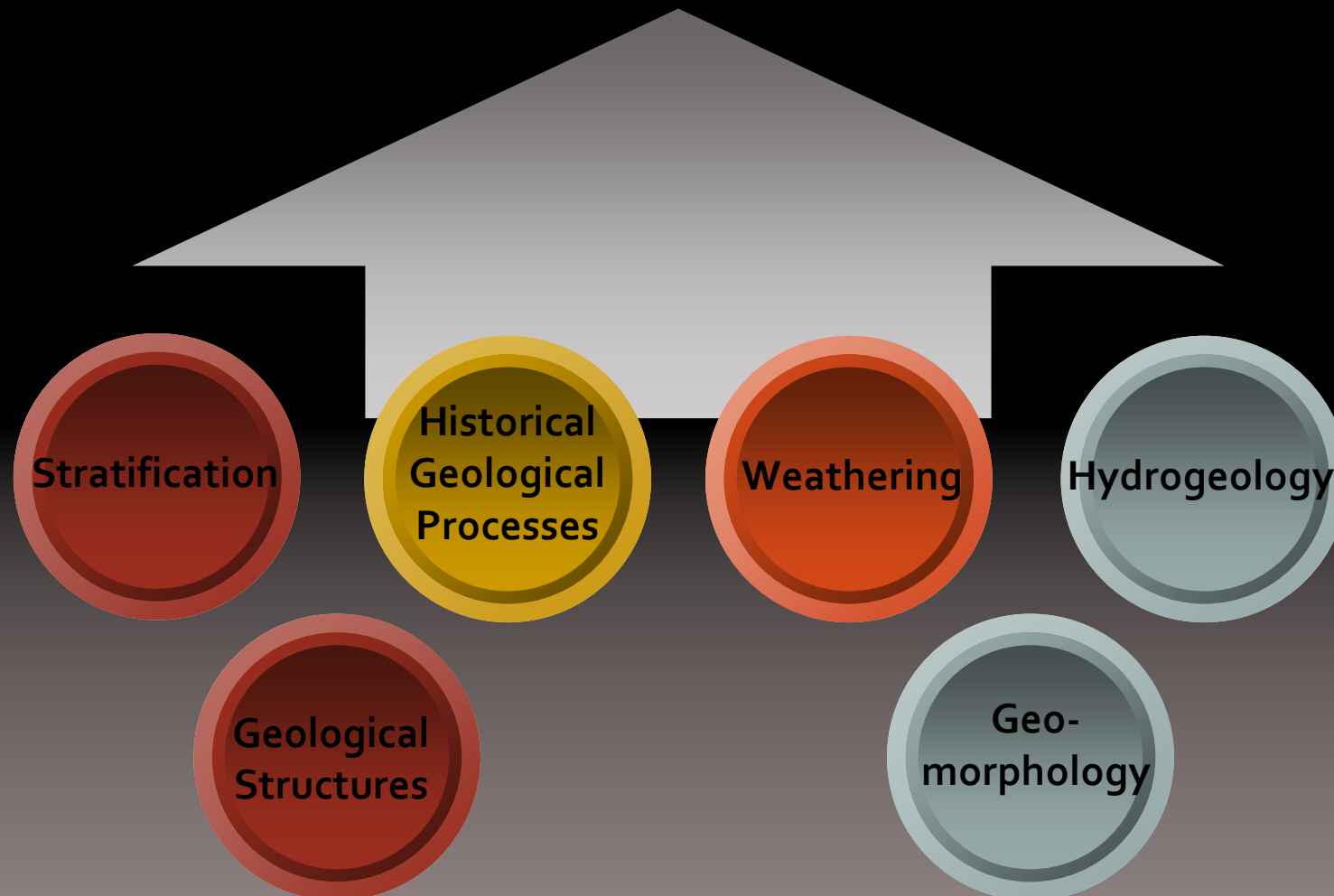
• Consulting • Technology • Outsourcing

Source : Life Style Magazine - EDGE



Ground Characterisation

Focus of **Geological** Model



Geological Mapping

Mapping of :

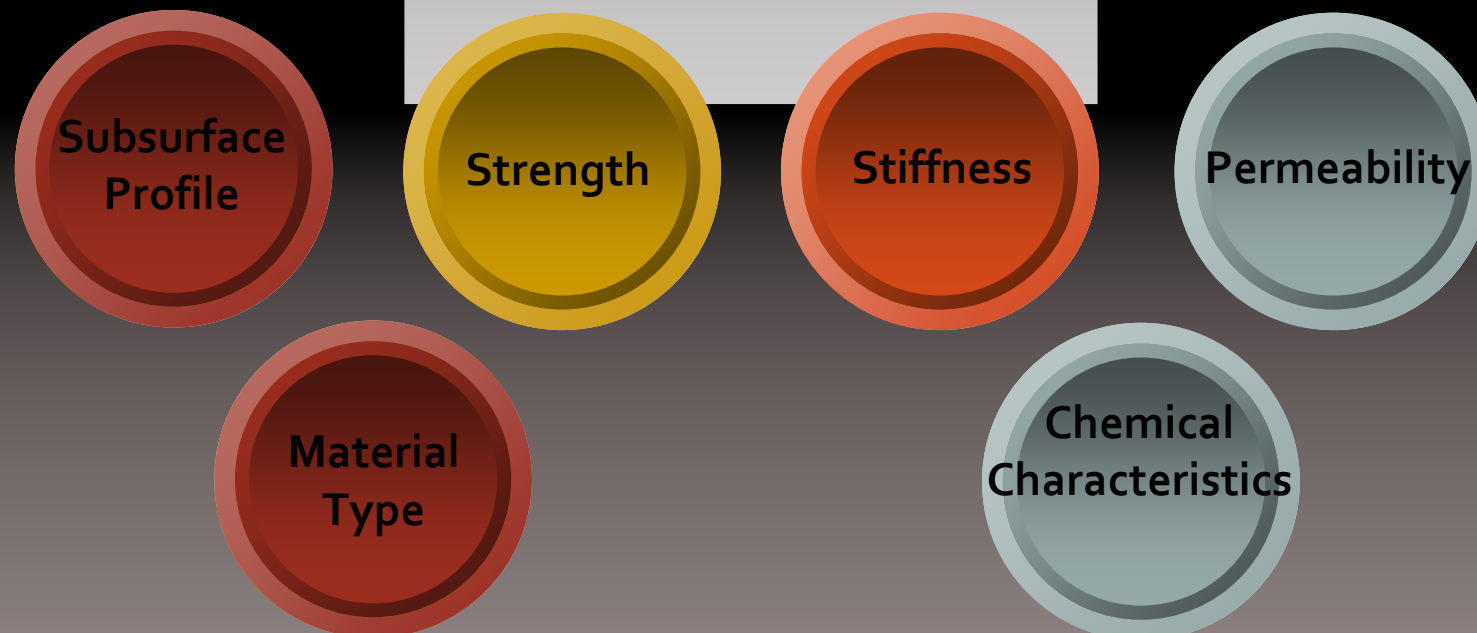
- Geological features (Structural settings)
- Weathering profile
- Outcrop exposure
- Seepage conditions

- Geomorphology
- Lithology
- Stratification
- Sequence of geological actions & history

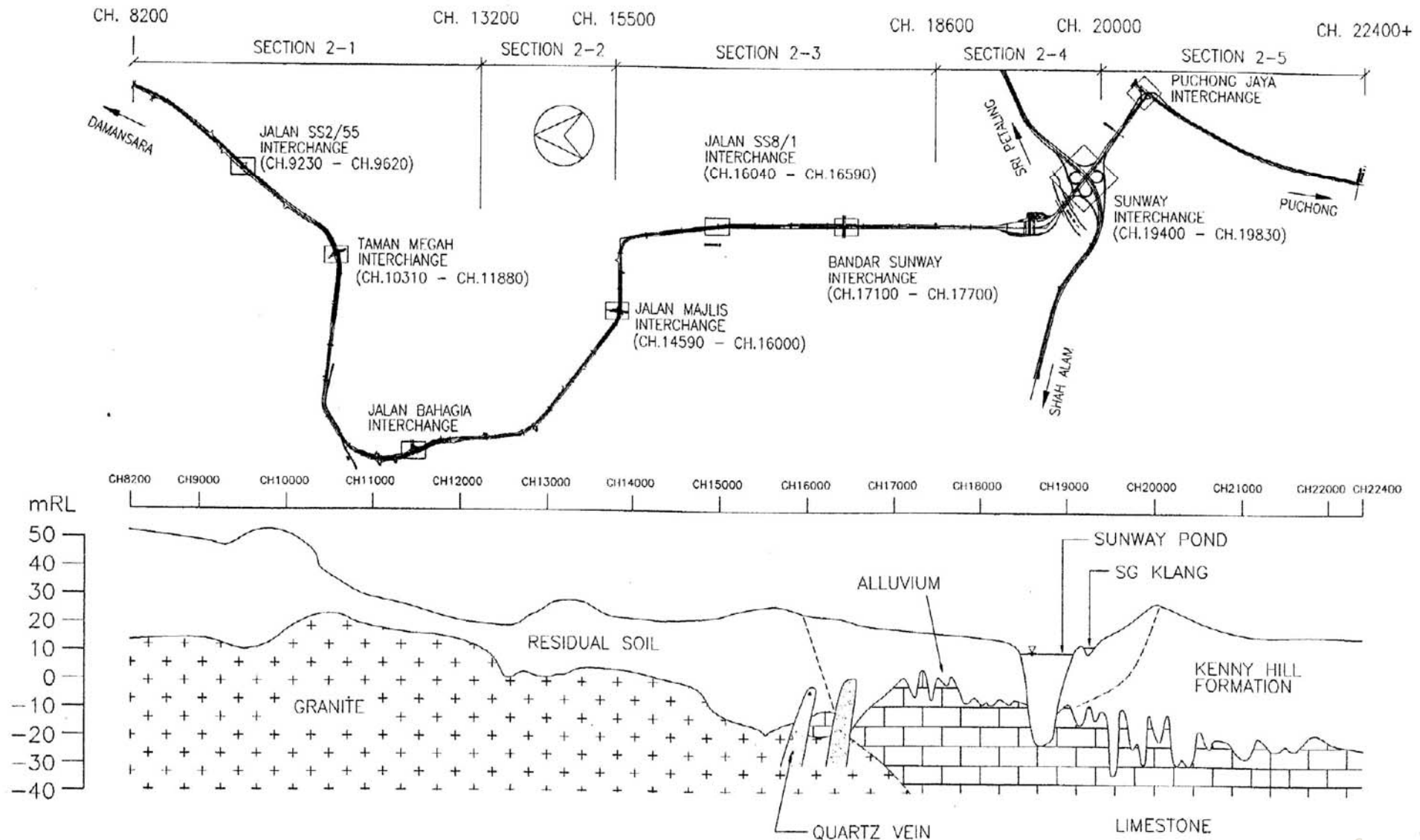


Ground Characterisation

Focus of **Geotechnical** Model



GEOTECHNICAL MODEL





COMMON PROBLEMS & TREND

General Dilemma of GI Industry

- Lack of pride & appreciation from consultant/client in GI industry.
- Actions done is considered work done!
Poor professionalism.
- Financial survival problem due to competitive rates in uncontrolled environment (cutting corner)
- No appropriate time frame for proper work procedures (shoddy works)
- Shifting of skilled expert to Oil & Gas or other attractive industries

■ Poor Planning & Interpretation

- Inadequate investigation coverage vertically & horizontally
- Wrong investigating tools
- No/wrong interpretation
- Poor investigating sequence



|| Poor Site Implementation

- Lack of level & coordinates of probing location
- Sample storage, handling, transportation
- Inappropriate equilibrium state in Observation Well & Piezometer

■ Poor In-situ & Laboratory Results

- Lack of equipment calibration
 - Wear & Tear Errors
 - Equipment systematic error (rod friction, electronic signal drift, unsaturated porous tip)
 - Defective sensor
 - Inappropriate testing procedures
- Equipment calibration (Variation of pH Values)
 - Improper sample preparation
 - Inadequate saturation
 - Inappropriate testing rate
 - Inadequate QA/QC in testing processes
 - Inherent sample disturbance before testing

|| Poorly Maintained Tools



|| Over-confidence in Geophysics

- We detect everything in geophysical data, but indentify almost nothing (**Rich** but **Complex**).
- Not a unique solution in tomographic reconstruction
(Indirect method)
- Poor remuneration to land geophysicist as compared to O&G
- Poor investigation specification
- Lack of good interpretative skill (human capital)
- High capital costs in equipment & software investment

Communication Problem



Difficulties in Identification of Complex Geological Settings



Difficulties in Identification of Complex Geological Settings



Weathering Profile

- Deviation of material classification between borehole and excavation

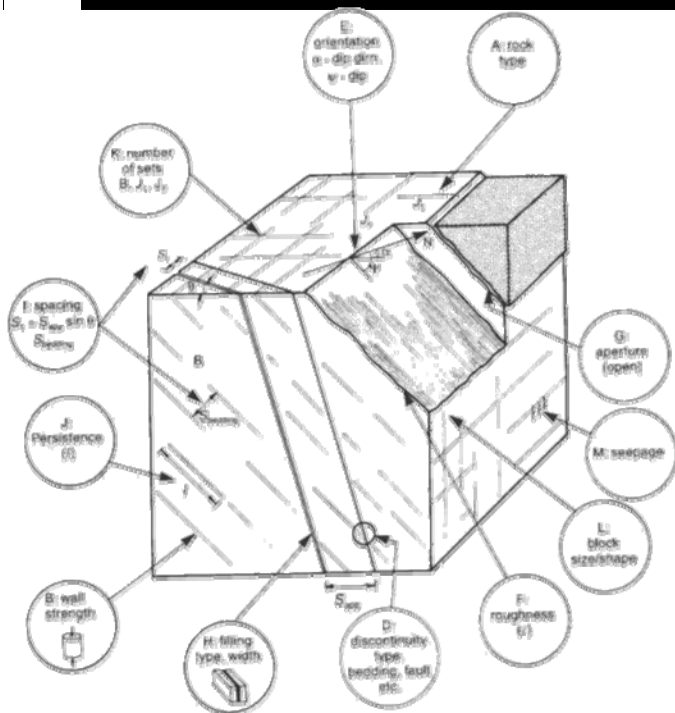
(Claim issue –
Soil or Rock?)



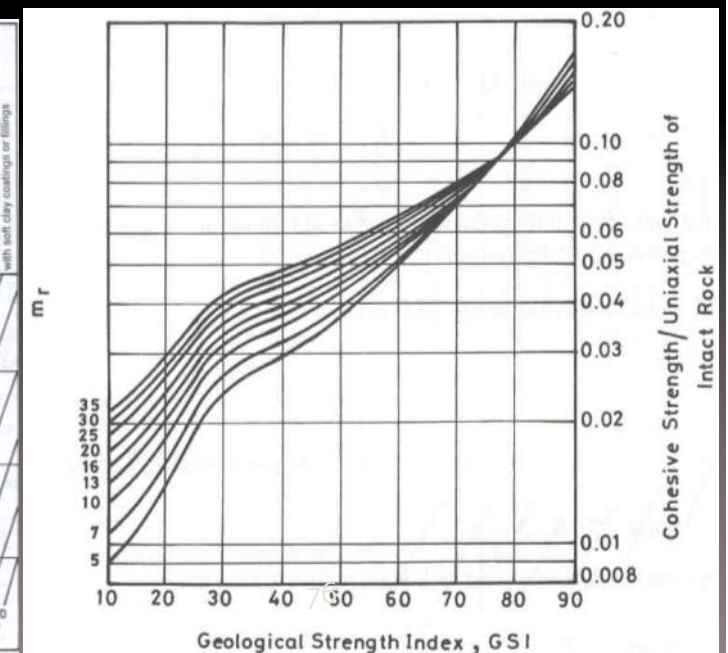
Complexity of Rock Mass

- **Properties**
- Complexity of rock mass strength (slope & excavation design)
- Empiricism requiring judgement (involving subjectivity)
- Information normally only available during construction, not design stage

$$\sigma_1' = \sigma_3' + \sigma_u \left[m_b \left(\frac{\sigma_3'}{\sigma_u} \right) + s \right]^a$$



GEOLOGICAL STRENGTH INDEX	
According to geological conditions, pick the appropriate box in this chart. Estimate the average value of the Geological Strength Index GSI from the contours.	
STRUCTURE	DISCONTINUITY SURFACE CONDITION
BLOCKY - very well interlocked undisturbed rock mass consisting of cubical blocks formed by three orthogonal discontinuity sets	VERY GOOD Very rough, unweathered surfaces
VERY BLOCKY - interlocked, partially distributed rock mass with multifaceted angular blocks formed by four or more discontinuity sets	GOOD Rough, slightly weathered, iron stained surfaces
BLOCKY/FOLDED - folded and faulted with many intersecting discontinuities forming angular blocks	FAIR Smooth, moderately weathered or altered surfaces
CRUSHED - poorly interlocked, heavily broken rock mass with a mixture of angular and rounded blocks	POOR Slackened, highly weathered surfaces with soft clay coatings or fillings



Unexpected Blowout of Underground Gas

- Gas pockets at 32m bgl
- Flushing out of sand



Supervision

- Work compliance & certification
- Document critical information
- Timely on-course instruction (sampling, in-situ testing & termination)
- Checking between field records and reported information

Future Trend - Electronic Data Collection, Transfer & Management

- AGS data transfer format & AGS-M format (monitoring data)

- First Edition in 1992, AGS(1992)
- Second Edition in 1994, AGS(1994)
- Third Edition in 1999

- Advantages :

- Efficient & Simplicity
- Minimised human error
- GI & Monitoring Data Management System
- Record keeping
- Spatial data analysis

The screenshot shows the 'Monitoring Points BH1001' window. It features a table with columns: Distance, ID, Date, and Type. Below the table are input fields for Depth (2.300), ID (1), and Type (SPIE). To the right, there are fields for Installation Date (12/09/2003) and a 'Response Zone' section with 'Top' (2.400) and 'Base' (6.700) values. Below this is a 'Drawing Details for Log' section with 'Tip Size' (200 mm) and checkboxes for 'Slotted Pipe' and 'Cross Hatching'. At the bottom, there are tabs for 'Axis Bearings', 'Axis Inclinations', 'Conventions', and 'Remarks', with input fields A, B, and C. A 'Documents...' button is also present. At the very bottom are 'Edit', 'New', 'Delete', and 'Done' buttons.

Distance	ID	Date	Type
2.300	1	12/09/2003	SPIE
5.800	2	-	-

Depth: 2.300 Installation Date: 12/09/2003
ID: 1 Response Zone: Top: 2.400, Base: 6.700
Type: SPIE
Drawing Details for Log: Tip Size: 200 mm, ☒ Slotted Pipe, ☒ Cross Hatching
Axis Bearings: A: -, B: -, C: -
Buttons: Edit, New, Delete, Done, Documents...

Conclusions

- Nature of GI works & Geotechnical design (**Uncertainties**)
- Role of Geotechnical Engineer, Engineering Geologist & Geophysicist
- Stages of GI works (**Planning, Implementation, Interpretation & Report**)
- Specifications
- Methodology of GI (Merits & Demerits)
 - Fieldworks (Direct/Indirect) + Geological Mapping
 - Laboratory tests
- Common Problems & Future Trend

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THANKYOU



SITE RECONNAISSANCE



Case Study 1 - Geotechnical Review

- The underlying soils are mainly **soft & compressible soils**
- Characteristics:
 - Compressible
 - Settling under loading (eg. fill) with time

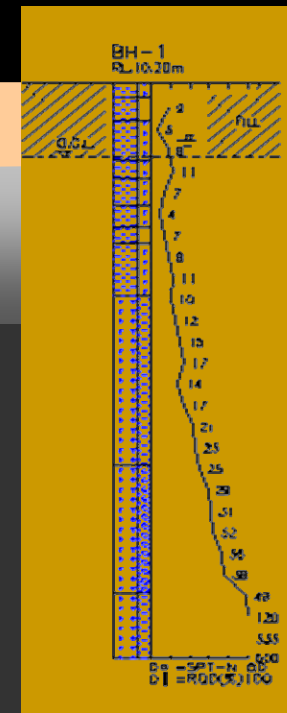
General Subsoil Profile

Blocks 1, 3, 17 (P. 2C), Blocks 1, 2 (P. 2D)

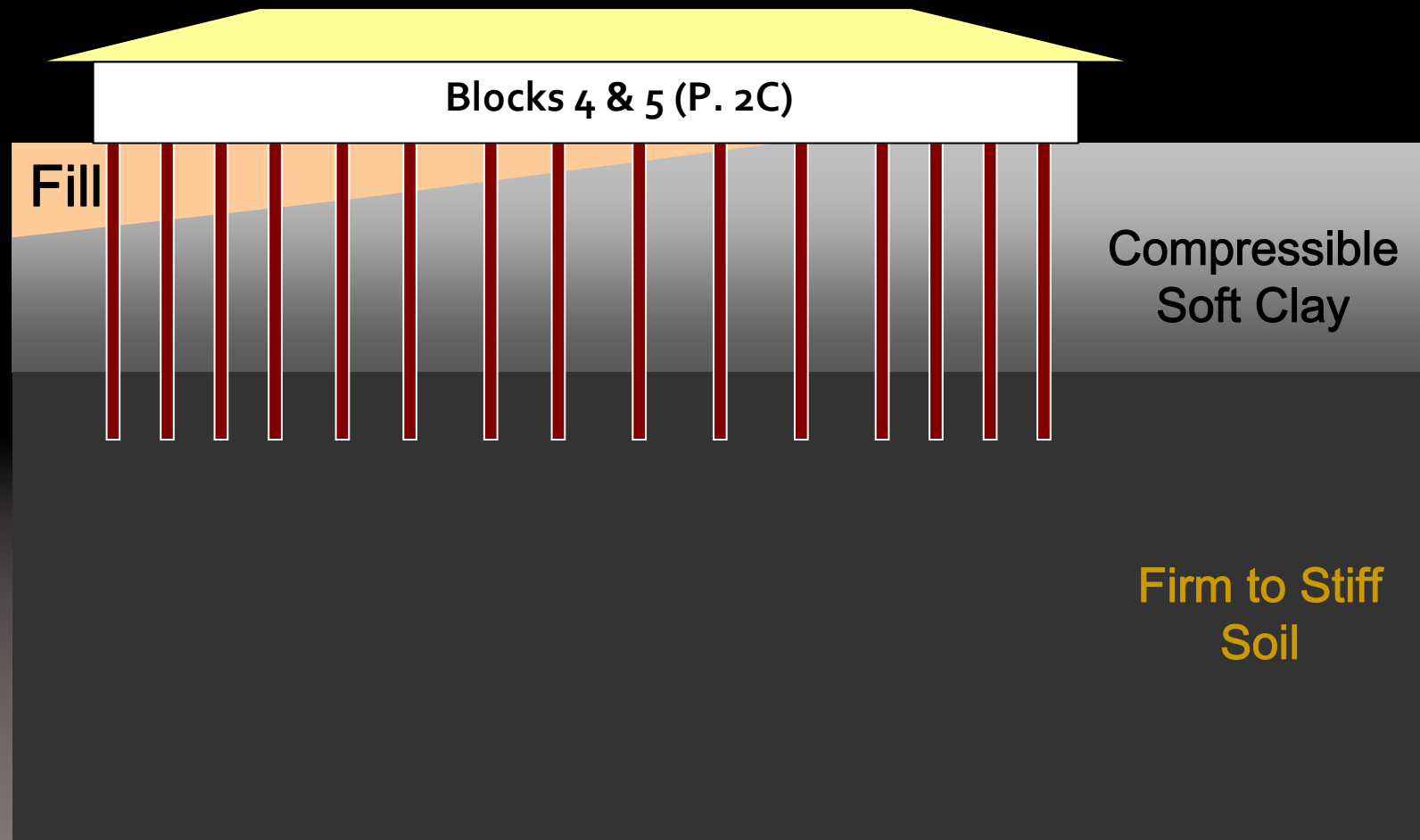
Fill

Compressible
Soft Clay

Firm to Stiff
Soil



General Subsoil Profile



Site Observations

- Cracks on wall – mostly diagonal – due to differential settlement



Site Observations

- Distress due to differential settlement



Probable Causes

1. Collapse settlement of unsaturated fill

- Occurs when saturation of loose fill (eg. during raining)
- S.I. results confirmed existence of fill at most areas

Probable Causes

2. Long term settlement of compressible soft soil

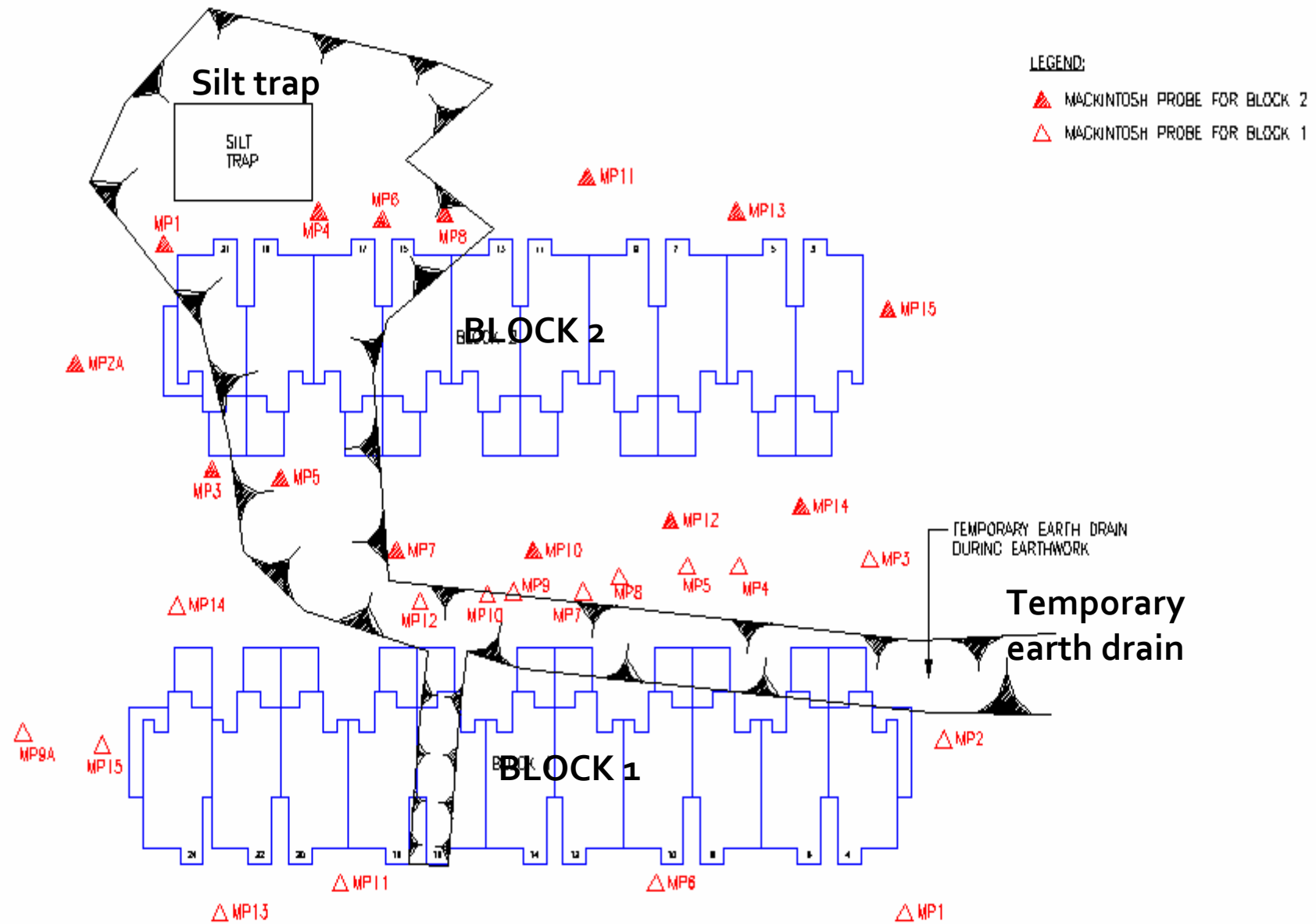
- Occurs when filling over soft soil
- S.I. results confirmed existence of soft soil

Probable Causes

3. Left-over soft deposits within silt trap & temporary drains

- ▣ Results in localised soft spots – more compressible
- ▣ Additional S.I. results confirmed existence of soft soil

Phase 2D1



Probable Causes

Subsoil settlement



Additional drag load on pile



Pile settlement



Differential settlement due
to different load, support,
fill & soft soil thickness



Distress on Structures

Remedial Works by Specialist Contractor

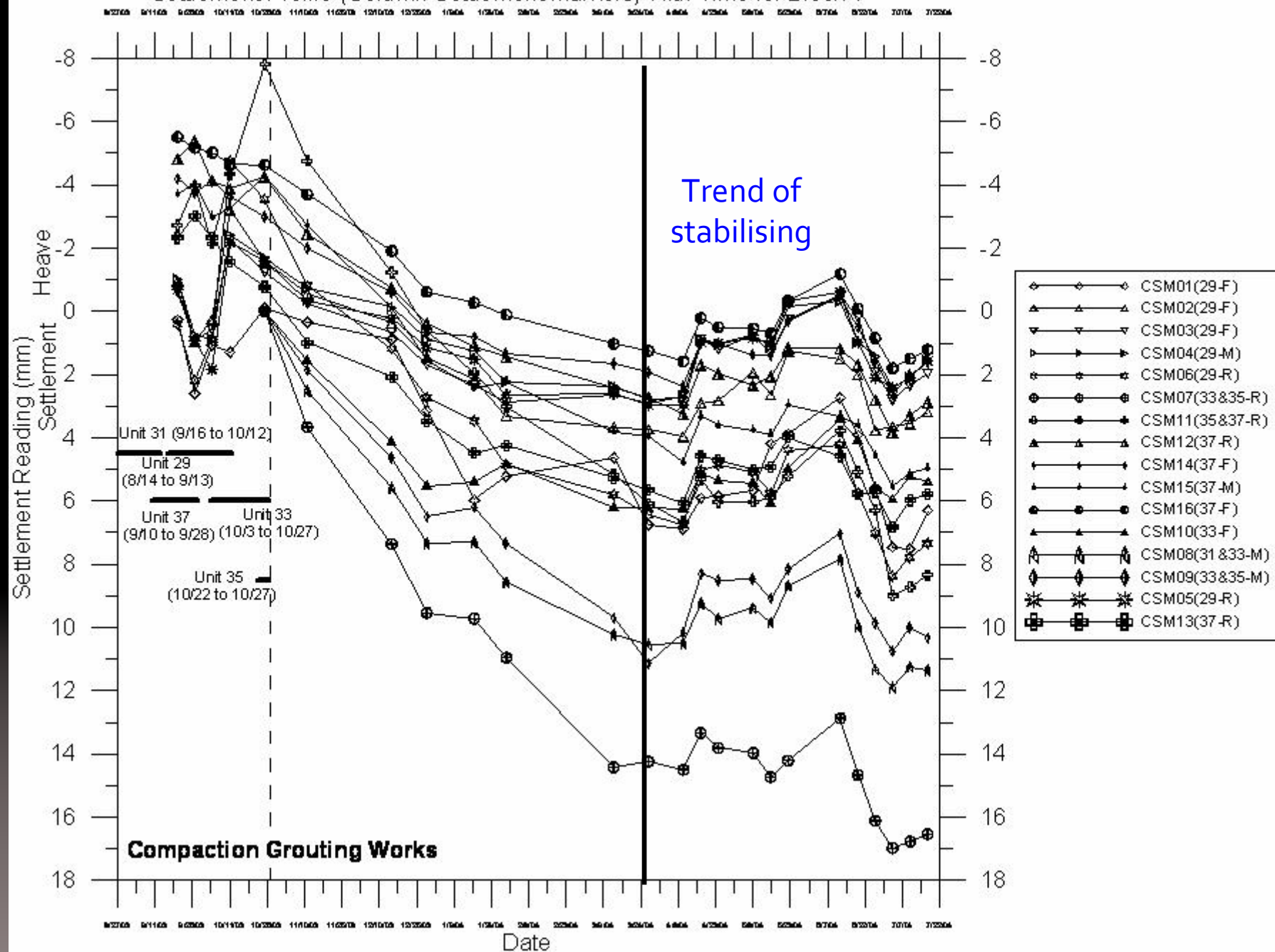
- **Grouting** has been carried out by specialist contractor at Block 1 of Phase 2C2
- Purpose: Fill in voids and densify compressive soft soil to eliminate ground settlement



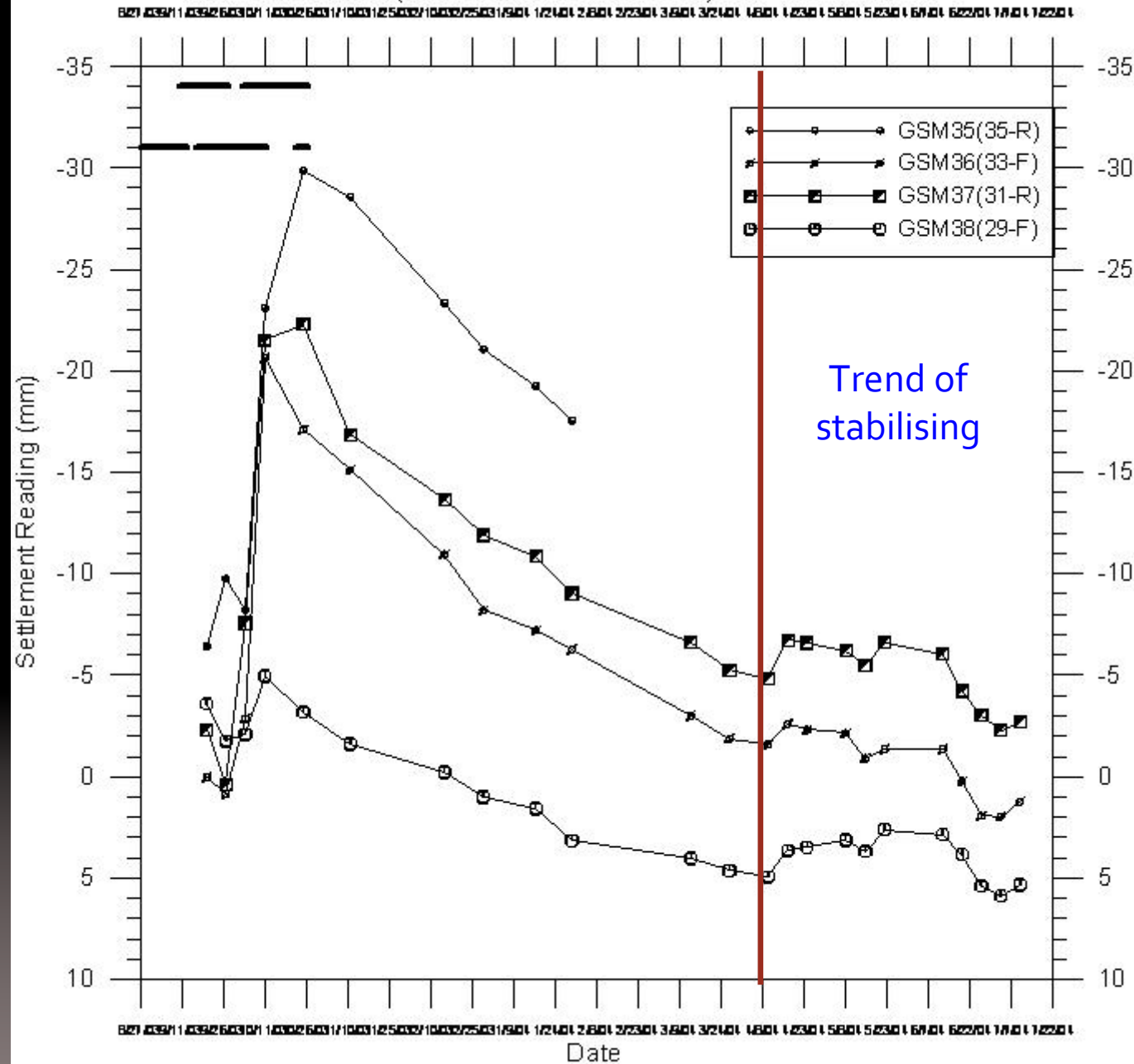
Remedial Works by Specialist Contractor

- Settlement is stabilising after grouting treatment

Settlement Profile (Column Settlement Markers) With Time for Block 1




Settlement Profile (Ground Settlement Marker) With Time for Block 1



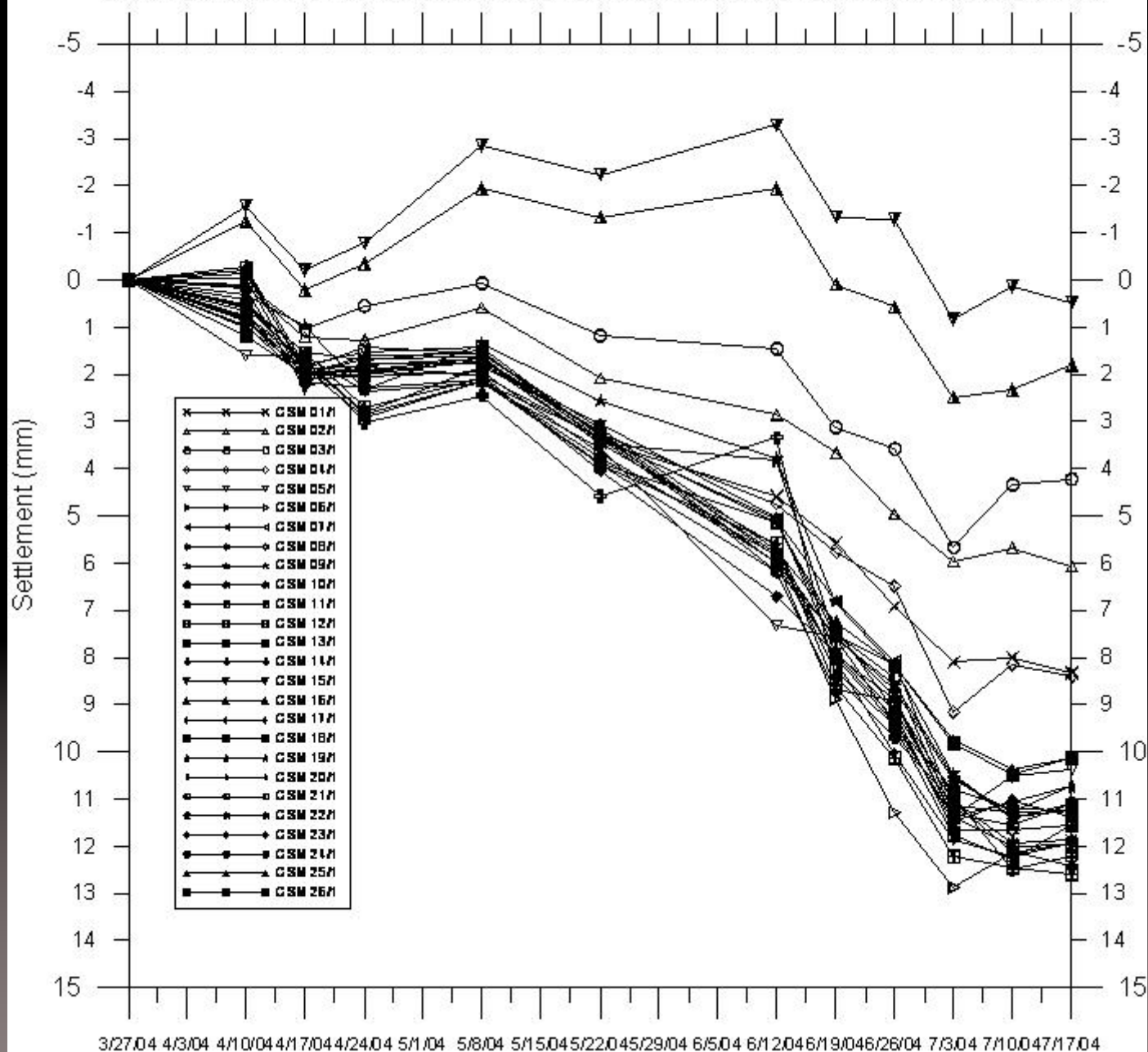


Monitoring Results

- Crack monitoring (3 months)
 - Settlement monitoring (10 months)
 - ▣ Ground settlement
 - ▣ Column settlement
- 

Settlement Profile (CSM) With Time for Block 1 (Phase 2D)

3/27/04 4/3/04 4/10/04 4/17/04 4/24/04 5/1/04 5/8/04 5/15/04 5/22/04 5/29/04 6/5/04 6/12/04 6/19/04 6/26/04 7/3/04 7/10/04 7/17/04



Settlement Profile With Time for Block 1 (Phase 2D)

